



# On-Site Engineering INC.

*Civil & Environmental Engineering*

Registered in CT, RI & MA

April 25, 2022

Mr. Jason Parker  
Principle Planner  
Town of South Kingstown  
180 High Street  
South Kingstown, Rhode Island 02879

**RE: Drainage Calculations, Minor Subdivision Application**  
**Applicant: Lee G. Hemmerle, Trust**  
**AP 74, Parcel 7**  
**350 Ministerial Road, South Kingstown, RI**

Dear Mr. Parker:

Per requirements of the Preliminary Checklist for a Minor Subdivision On-Site Engineering, Inc. (OSE) has conducted drainage calculations to support a new residential house lot located at 350 Ministerial Road in South Kingstown. The existing lot is approximately 7.9 acres and contains an existing single-family dwelling located approximately 300 feet westerly from Ministerial Road. This minor subdivision application will involve the creation of two lots. Lot 1 will be 2.97 acres and contain the existing house and paved access driveway. Lot 2 will measure 4.92 acres and will be located at the rear of the Subject Parcel. In order to access Lot 2, an 850-foot-long driveway consisting of both gravel and bituminous pavement will be constructed along the southerly boundary line. Grassed swales are also proposed along north and south sides of the driveway

The BMP's were designed per requirements of the *State of Rhode Island Stormwater Management Guidance for Individual Single-Family Residential Lot Development*. The following are the narrative and design calculations for the stormwater BMP's.

## **Watershed Description**

The Subject Property is divided into three watersheds. The limits of DA-1 is approximately 3.63 acres and follows an existing ridge line located approximately 680 feet from Ministerial Road and discharges to a natural low point. DA-1 contains the existing residential house and paved driveway. DA-2 is approximately 2.85 acres and also follows an existing ridge line located just east of the westerly property line. DA-2

discharges to the south off site. DA-3 is approximately 0.56 acres and discharges to a natural low point at the southeast corner of the Subject Parcel. DA-1 is a developed residential lot while DA-2 and DA-3 is heavily wooded with slopes ranging from 5-35%. The surrounding land use consists of developed residential lots, undeveloped wooded areas and roadways.

Table 1 summarizes the pre-development watershed characteristics as well as the runoff generated for the 2, 10, 25 and 100 year storm events.

**Table 1: Summary of Input Parameters – Existing Conditions**

<b>SubArea</b>	<b>Area (acres)</b>	<b>CN</b>	<b>Tc (Min.)</b>
DA-1	3.63	34	20
DA-2	2.85	30	41
DA-3	0.56	34	18

**Table 2: Summary of HydroCadd Output Results, Total Offsite – Existing Conditions**

<b>Storm Event</b>	<b>DA-1</b>		<b>DA-2/DA-3-Total Offsite</b>	
	<b>Peak Flow (cfs)</b>	<b>Total Volume (ac-ft.)</b>	<b>Peak Flow (cfs)</b>	<b>Total Volume (ac-ft.)</b>
2	---	---	---	---
10	0.02	0.015	---	---
25	0.12	0.069	0.04	0.030
100	1.39	0.268	0.44	0.170

Attachment 1 contains the HydroCadd input parameters as well as the output results for the existing conditions.

### ***Soils and Landuse***

The Subject Property contains Gloucester-Hinckley (GhD and GhC) soils. GhD and GhC soils are excessively drained to somewhat excessively drained soils located on hills and ridges. Typically, Gloucester/Hinckley soils have a surface layer of dark brown gravelly sandy loam about 2-inches thick. The subsoil is approximately 20-inches thick and consists of a brownish yellow to light yellowish gravelly sandy loam. The substratum is light brownish gray very gravelly loamy sand to a depth of 60-inches and greater. The permeability of GhD and ChC soils is rapid in the surface and subsoil and very rapid in the substratum with engineering properties of 6-20 inches per hour.

## Water Quality

The BMP's were designed to treat the runoff generated from the proposed impervious areas. The proposed driveway will discharge to grassed swales with check dams and the roof area for the proposed house will be directed towards underground infiltration practices for water quality treatment.

The drainage calculations were conducted with the driveway as an impervious surface. The driveway is within DA-2A and DA-2B with a total area of 10,200 s.f (0.23 acres)

### *Water Quality Calculations*

The impervious area associated with the proposed roofed area is 3,240 s.f. and will be collected and allowed to infiltrate into four UIC's consisting of a 5'x12' stone trench, 36-inches deep. The proposed driveway will be graded in a manner as to direct runoff into grassed swales along the edges of the driveway. The swales will have stone check dams to allow infiltration and reduce the velocity within the channel. The swales measure 1 foot deep, 1 foot wide with 3:1 side slopes. Based upon an impervious area of 10,200, the minimum surface area of the bio-retention swale is 245 s.f. Based upon a wetted perimeter of 2.2 feet and the total length of the swale of 850 feet, the area of the swale is 1,870 s.f. which is much greater than 245 s.f. that is required. Attachment 2 contains the both the stormwater and water quality calculations for the grassed swales.

## Conveyance and Natural Channel Protection

Due to the proposed driveway, the drainage areas for the developed conditions were divided into four drainage areas. DA-1 remains the same as the existing conditions, which contains the existing single family dwelling. The contributing drainage area is 2.43 acres, which is less than the 3.63 acres for the existing conditions. Again, this is due to the proposed driveway redirecting runoff from DA-1 to DA-2A. DA-2A contributes area to the swale located on the north side of the driveway. DA-2B follows the same ridge along the westerly property line and contributes to the swale located on the south side of the driveway. Both DA-2A and DA-2B ultimately discharges to the southeasterly corner of the Subject Parcel. DA-2C contributes off-site at the southwesterly corner of the Subject Parcel. Table 3 summarizes the input parameters for the developed conditions.

**Table 3: Summary of Input Parameters – Developed Conditions**

<b>SubArea</b>	<b>Area (acres)</b>	<b>CN</b>	<b>Tc (Min.)</b>
DA-1	2.43	35	18
DA-2A	1.20	37	50
DA-2B	0.60	41	38
DA-2C	0.93	32	18


The drainage calculations were conducted to illustrate that there is no increase in runoff from the 100-year storm event for the proposed single family residential lot. The pre-development 100-year discharge for DA-2/DA-3 was calculated as 0.44 cfs, while the post development 100-year discharge for DA-2A/DA-2B was calculated as 0.43 cfs. The BMP's that were implemented to control the peak runoff was bioretention swales with check dams and directing the runoff to an infiltration area located at the beginning of the driveway. The check dams within the grassed swales allow infiltration, slow the velocities to an acceptable value to prevent erosion and increase the overall time of concentration by "daming" the runoff behind the stone structures. Supporting drainage calculations are attached with this report. The infiltration area located at the beginning of the driveway provides both surface storage as well as underground storage. The subsurface storage/infiltration system consists of (4) 4'x4' concrete galleys with 12-inches of stone on the bottom and sides. For the extreme storm events an emergency spillway was incorporated into the design. Table 4 illustrates that the post development discharge for the 100-year storm event is 0.43 cfs, which is equivalent to the pre-development conditions. The runoff generated for the 2, 10 and 25-year storm events all are contained within the infiltration area.

**Table 4: Summary of HydroCadd Output Results, Total Offsite – Existing Conditions**

Storm Event	DA-1		DA-2A/DA-2B- Total Offsite	
	Peak Flow (cfs)	Total Volume (ac-ft.)	Peak Flow (cfs)	Total Volume (ac-ft.)
2	---	---	---	---
10	0.03	0.019	---	---
25	0.16	0.072	---	---
100	1.47	0.260	0.43	0.018

Attachment 3 contains the HydroCadd input parameters as well as the output results for the developed conditions and Attachment 4 contains the pre and post development drainage maps.

Thank you for your consideration and assistance in this matter. Should you have any questions or need additional information please feel free to call me at 401-348-6831.

Sincerely,  
  
 Anthony A. Nenna, P.E.  
 President

Attachments

cc: A. DiOrio

**ATTACHMENT 1**

**PRE-DEVELOPMENT DRAINAGE CALCULATIONS**



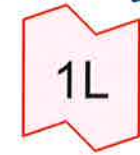
Drainage Area 1



Drainage Area 2



Drainage Area 3



Total Offsite



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Page 2

**Summary for Subcatchment DA-1: Drainage Area 1**

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

Area (ac)	CN	Description
2.880	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.630	34	Weighted Average
3.500		96.42% Pervious Area
0.130		3.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	115	0.1800	0.11		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	95	0.3800	3.08		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.5	125	0.0800	1.41		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
0.9	140	0.0280	2.69		<b>Shallow Concentrated Flow, Tt4</b> Unpaved Kv= 16.1 fps
19.7	475	Total			

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Pre-Development Drainage Analysis-350 Ministerial Rd

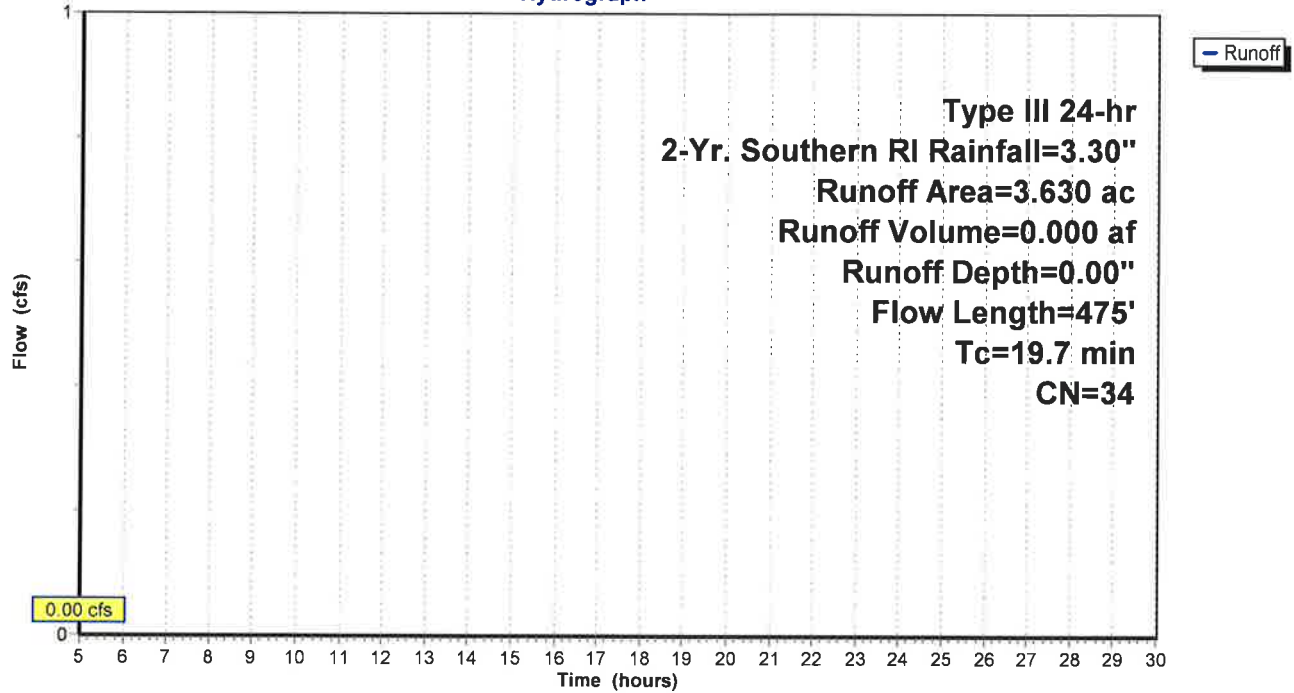
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

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Page 3

**Subcatchment DA-1: Drainage Area 1**

Hydrograph



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**Summary for Subcatchment DA-2: Drainage Area 2**

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

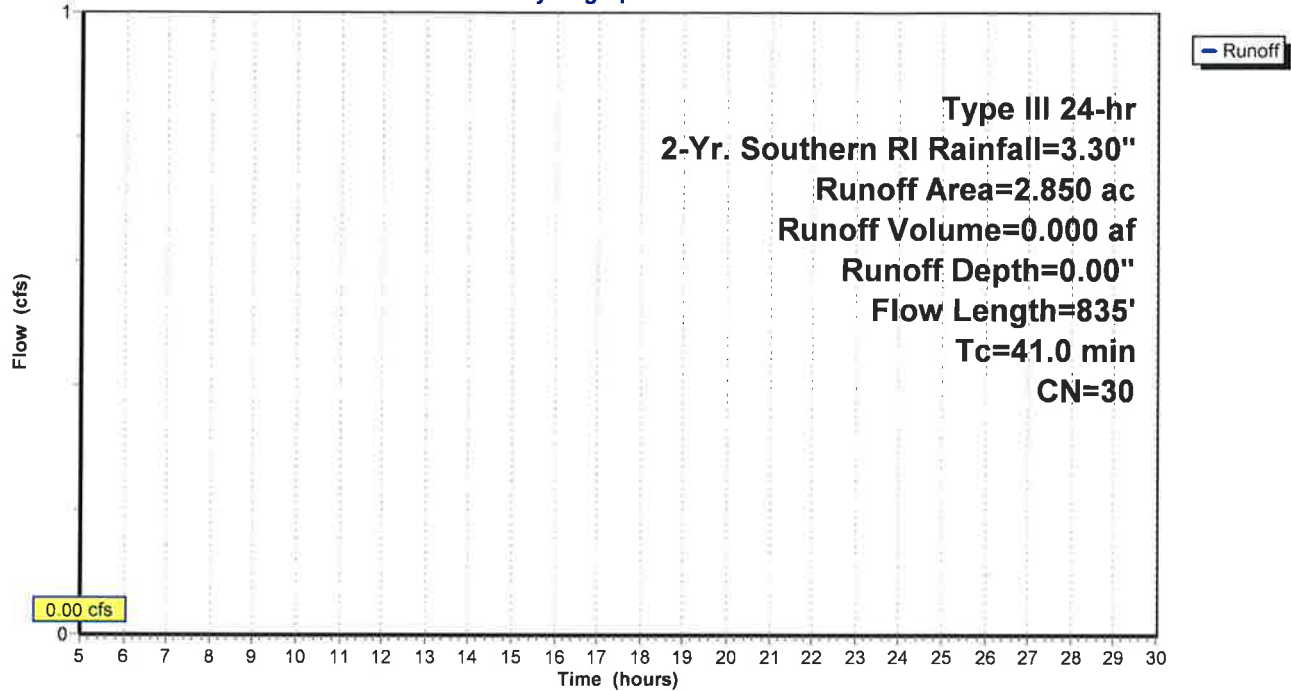
Area (ac)	CN	Description
2.850	30	Woods, Good, HSG A
2.850		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.5	155	0.1160	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.8	125	0.0480	0.55		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
6.8	345	0.0290	0.85		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
2.0	100	0.1100	0.83		<b>Shallow Concentrated Flow, Tt3</b> Forest w/Heavy Litter Kv= 2.5 fps
2.9	110	0.0640	0.63		<b>Shallow Concentrated Flow, Tt4</b> Forest w/Heavy Litter Kv= 2.5 fps
41.0	835	Total			

### Subcatchment DA-2: Drainage Area 2

Hydrograph



### Summary for Subcatchment DA-3: Drainage Area 3

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

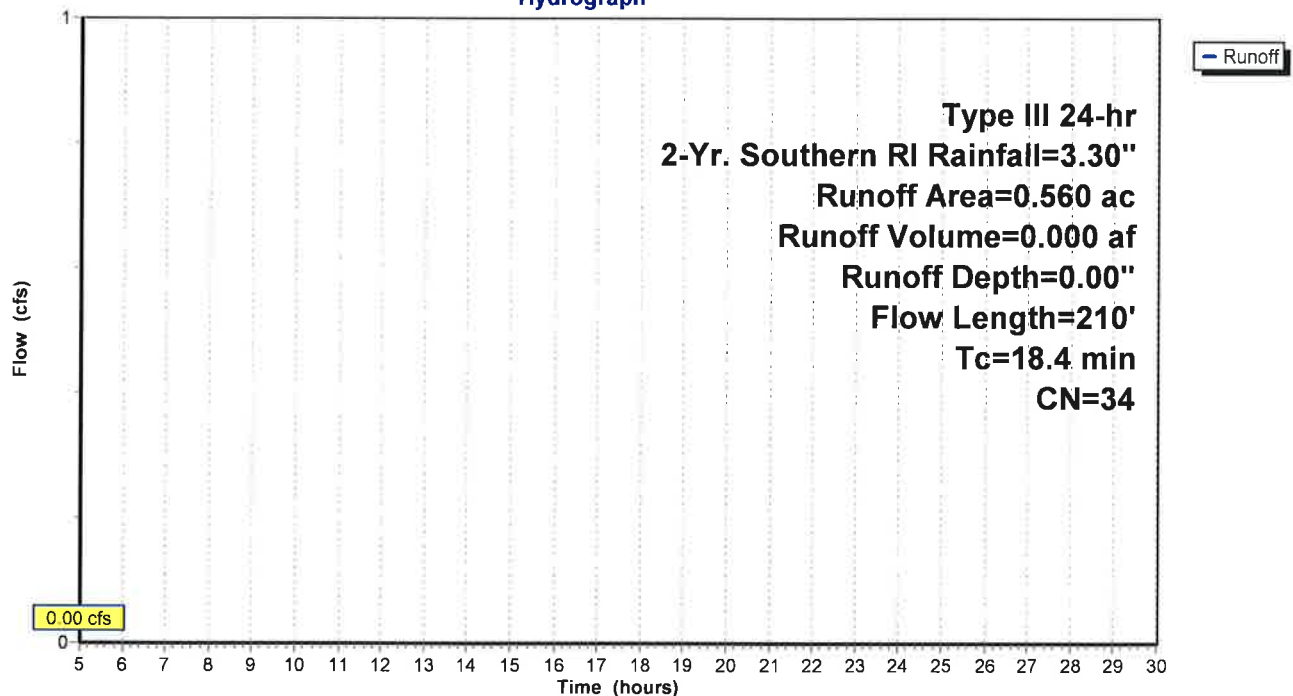
Area (ac)	CN	Description
0.510	30	Woods, Good, HSG A
0.030	98	Paved parking, HSG A
0.020	39	>75% Grass cover, Good, HSG A
0.560	34	Weighted Average
0.530		94.64% Pervious Area
0.030		5.36% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	110	0.0540	0.12		<b>Sheet Flow, Tt1</b>
					Woods: Light underbrush n= 0.400 P2= 3.40"
3.3	100	0.0100	0.50		<b>Shallow Concentrated Flow, Tt2</b>
					Woodland Kv= 5.0 fps
18.4	210	Total			

### Subcatchment DA-3: Drainage Area 3

Hydrograph



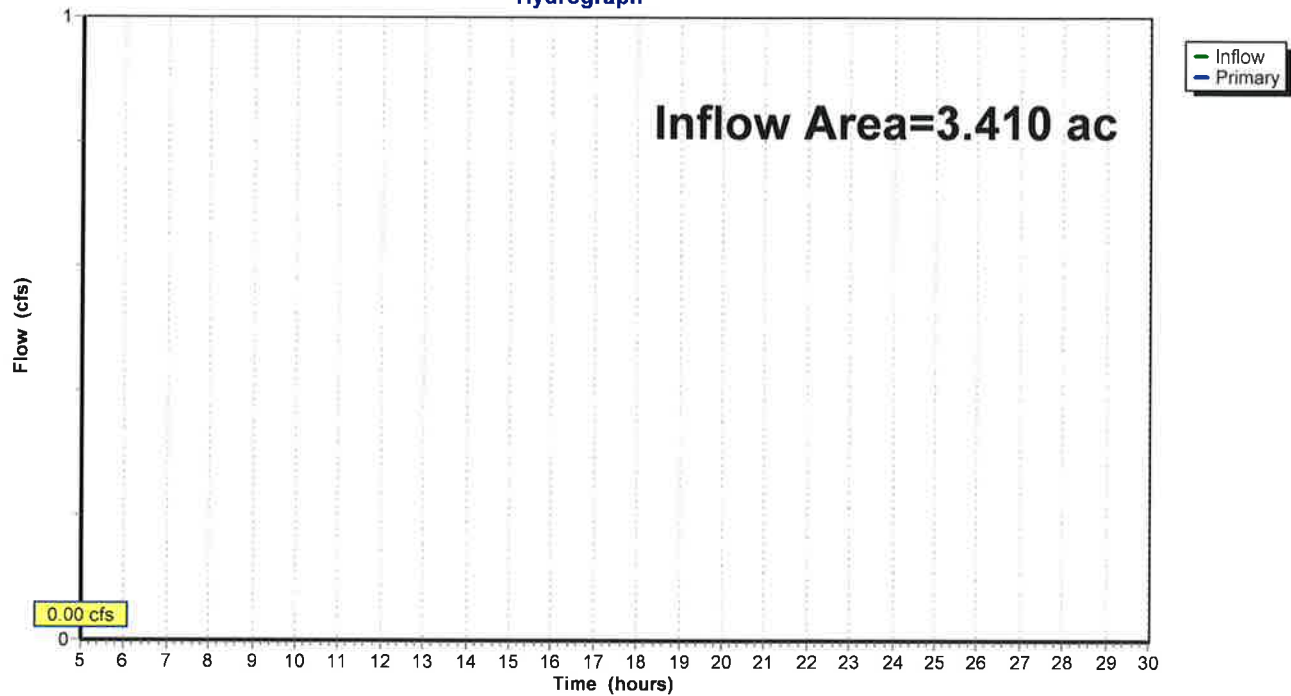
### Summary for Link 1L: Total Offsite

Inflow Area = 3.410 ac, 0.88% Impervious, Inflow Depth = 0.00" for 2-Yr. Southern RI event  
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

### Link 1L: Total Offsite

Hydrograph



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Page 8

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 0.02 cfs @ 15.87 hrs, Volume= 0.015 af, Depth= 0.05"

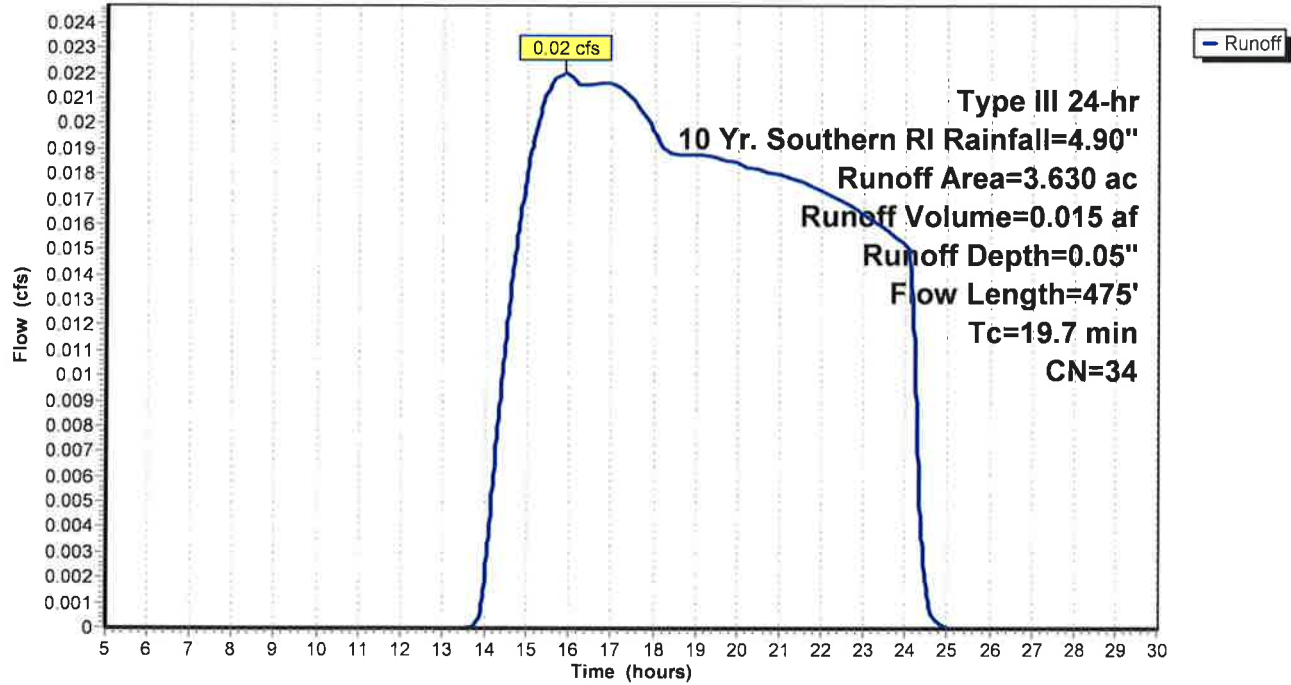
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

Area (ac)	CN	Description
2.880	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.630	34	Weighted Average
3.500		96.42% Pervious Area
0.130		3.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	115	0.1800	0.11		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	95	0.3800	3.08		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.5	125	0.0800	1.41		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
0.9	140	0.0280	2.69		<b>Shallow Concentrated Flow, Tt4</b> Unpaved Kv= 16.1 fps
19.7	475	Total			

### Subcatchment DA-1: Drainage Area 1

Hydrograph





### Summary for Subcatchment DA-3: Drainage Area 3

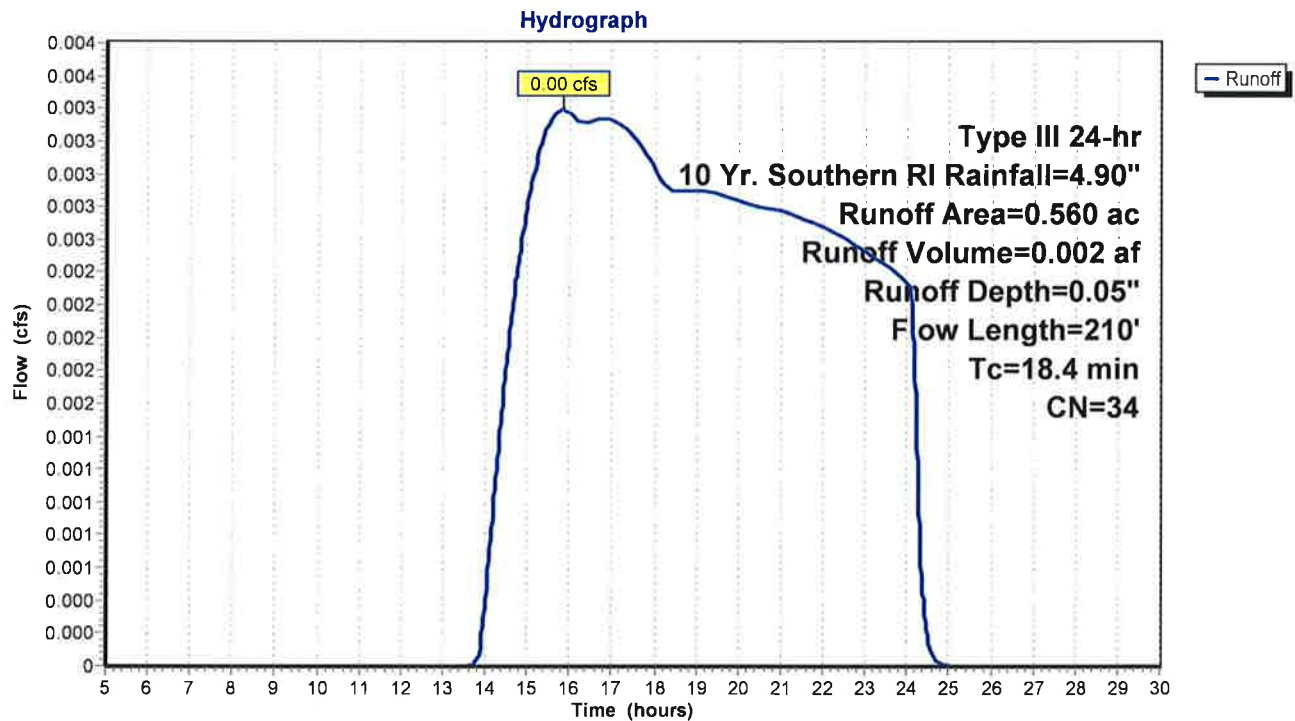
Runoff = 0.00 cfs @ 15.84 hrs, Volume= 0.002 af, Depth= 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

Area (ac)	CN	Description
0.510	30	Woods, Good, HSG A
0.030	98	Paved parking, HSG A
0.020	39	>75% Grass cover, Good, HSG A
0.560	34	Weighted Average
0.530		94.64% Pervious Area
0.030		5.36% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	110	0.0540	0.12		<b>Sheet Flow, Tt1</b>
					Woods: Light underbrush n= 0.400 P2= 3.40"
3.3	100	0.0100	0.50		<b>Shallow Concentrated Flow, Tt2</b>
					Woodland Kv= 5.0 fps
18.4	210	Total			

### Subcatchment DA-3: Drainage Area 3



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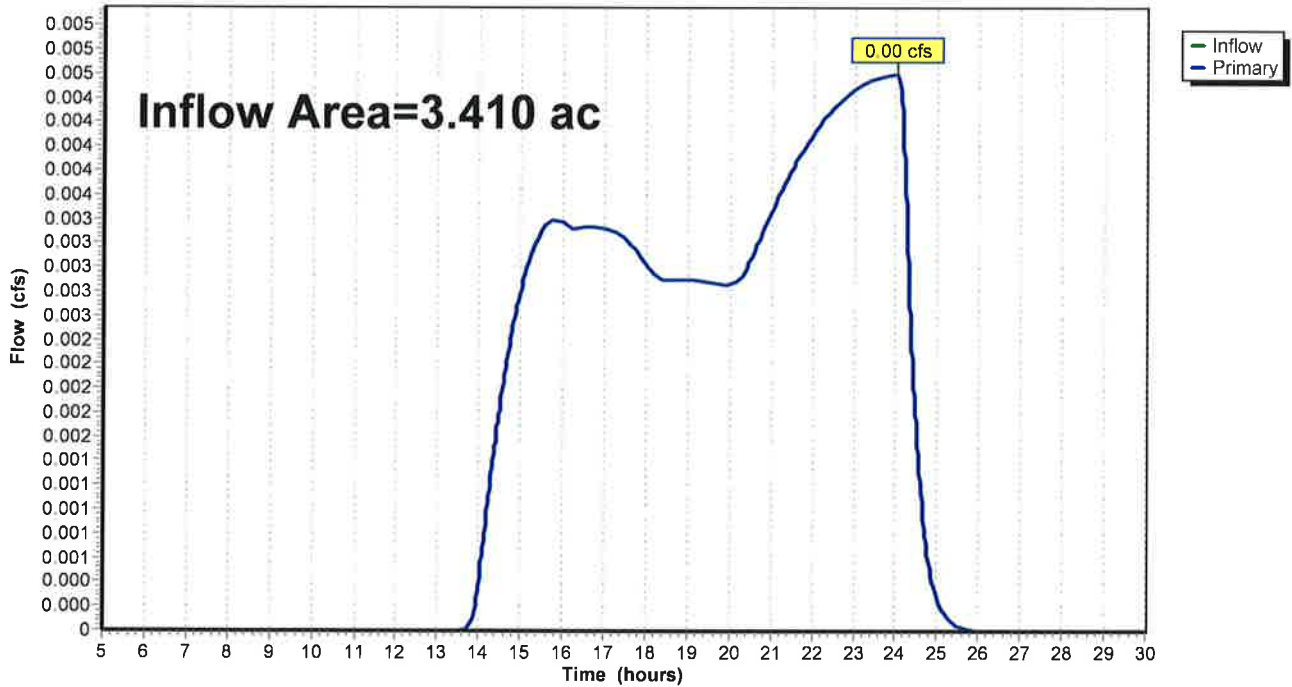
**Summary for Link 1L: Total Offsite**

Inflow Area = 3.410 ac, 0.88% Impervious, Inflow Depth = 0.01" for 10 Yr. Southern RI event  
Inflow = 0.00 cfs @ 24.02 hrs, Volume= 0.003 af  
Primary = 0.00 cfs @ 24.02 hrs, Volume= 0.003 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

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Page 13

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 0.12 cfs @ 13.77 hrs, Volume= 0.069 af, Depth= 0.23"

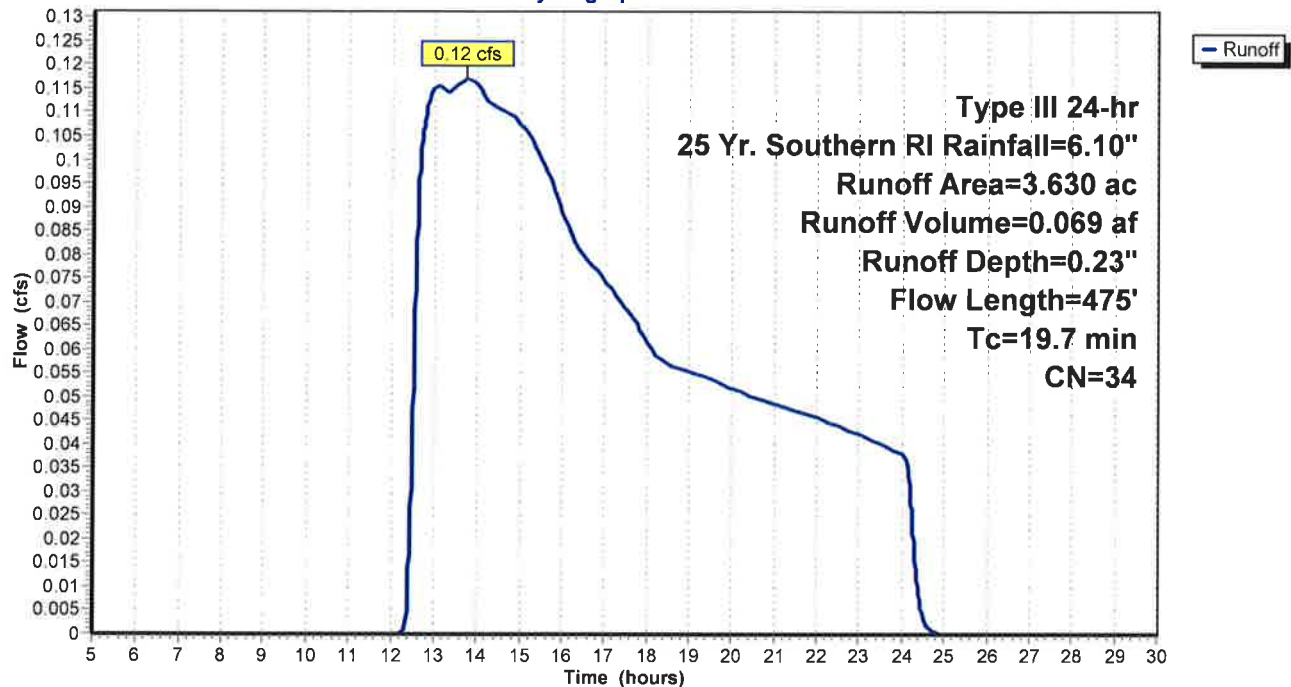
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

Area (ac)	CN	Description
2.880	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.630	34	Weighted Average
3.500		96.42% Pervious Area
0.130		3.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	115	0.1800	0.11		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	95	0.3800	3.08		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.5	125	0.0800	1.41		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
0.9	140	0.0280	2.69		<b>Shallow Concentrated Flow, Tt4</b> Unpaved Kv= 16.1 fps
19.7	475	Total			

### Subcatchment DA-1: Drainage Area 1

Hydrograph



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Pre-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

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Page 15

**Summary for Subcatchment DA-2: Drainage Area 2**

Runoff = 0.03 cfs @ 15.90 hrs, Volume= 0.020 af, Depth= 0.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

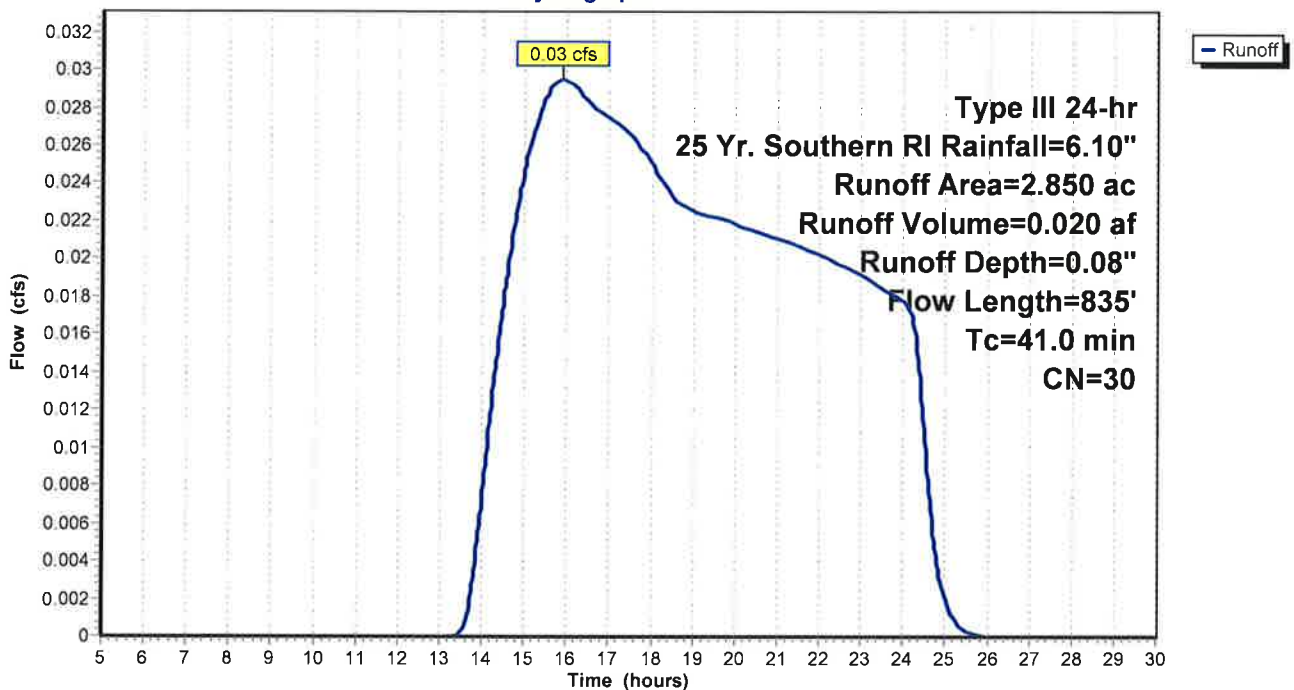
Area (ac)	CN	Description
2.850	30	Woods, Good, HSG A
2.850		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.5	155	0.1160	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.8	125	0.0480	0.55		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
6.8	345	0.0290	0.85		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
2.0	100	0.1100	0.83		<b>Shallow Concentrated Flow, Tt3</b> Forest w/Heavy Litter Kv= 2.5 fps
2.9	110	0.0640	0.63		<b>Shallow Concentrated Flow, Tt4</b> Forest w/Heavy Litter Kv= 2.5 fps
41.0	835	Total			

**Subcatchment DA-2: Drainage Area 2**

Hydrograph



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Page 16

**Summary for Subcatchment DA-3: Drainage Area 3**

Runoff = 0.02 cfs @ 13.76 hrs, Volume= 0.011 af, Depth= 0.23"

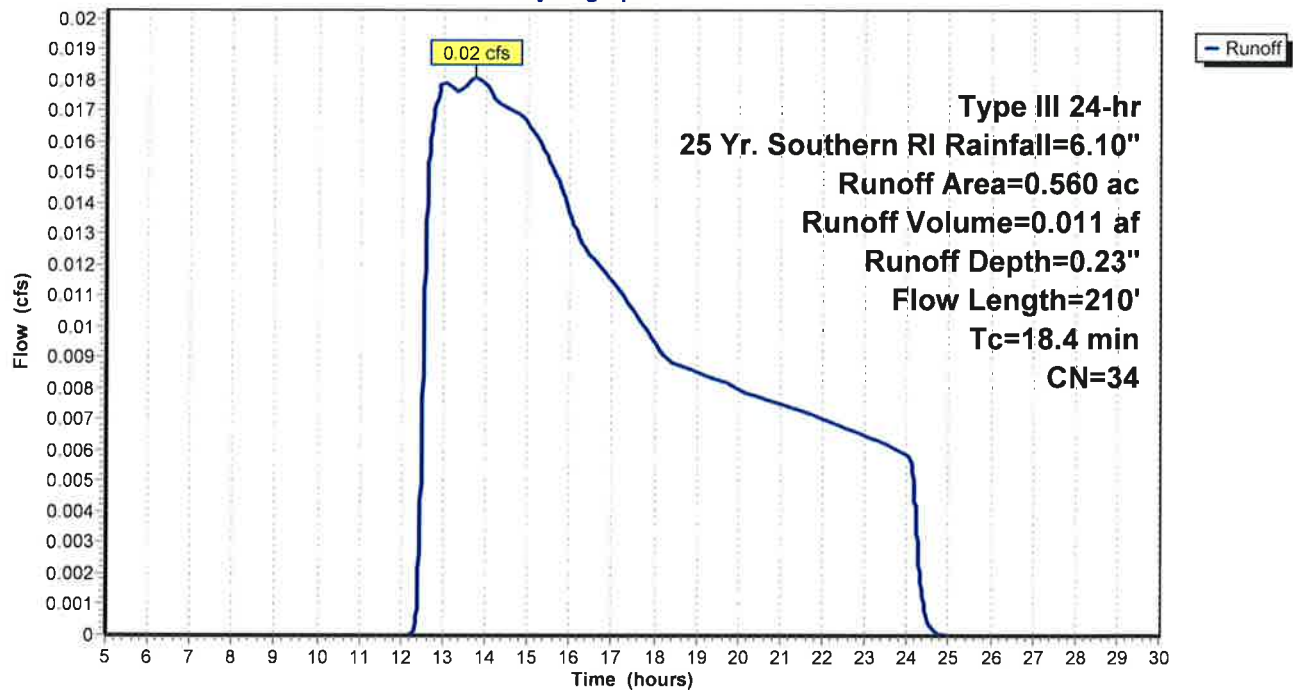
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

Area (ac)	CN	Description
0.510	30	Woods, Good, HSG A
0.030	98	Paved parking, HSG A
0.020	39	>75% Grass cover, Good, HSG A
0.560	34	Weighted Average
0.530		94.64% Pervious Area
0.030		5.36% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	110	0.0540	0.12		<b>Sheet Flow, Tt1</b> Woods: Light underbrush n= 0.400 P2= 3.40"
3.3	100	0.0100	0.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
18.4	210	Total			

**Subcatchment DA-3: Drainage Area 3**

Hydrograph



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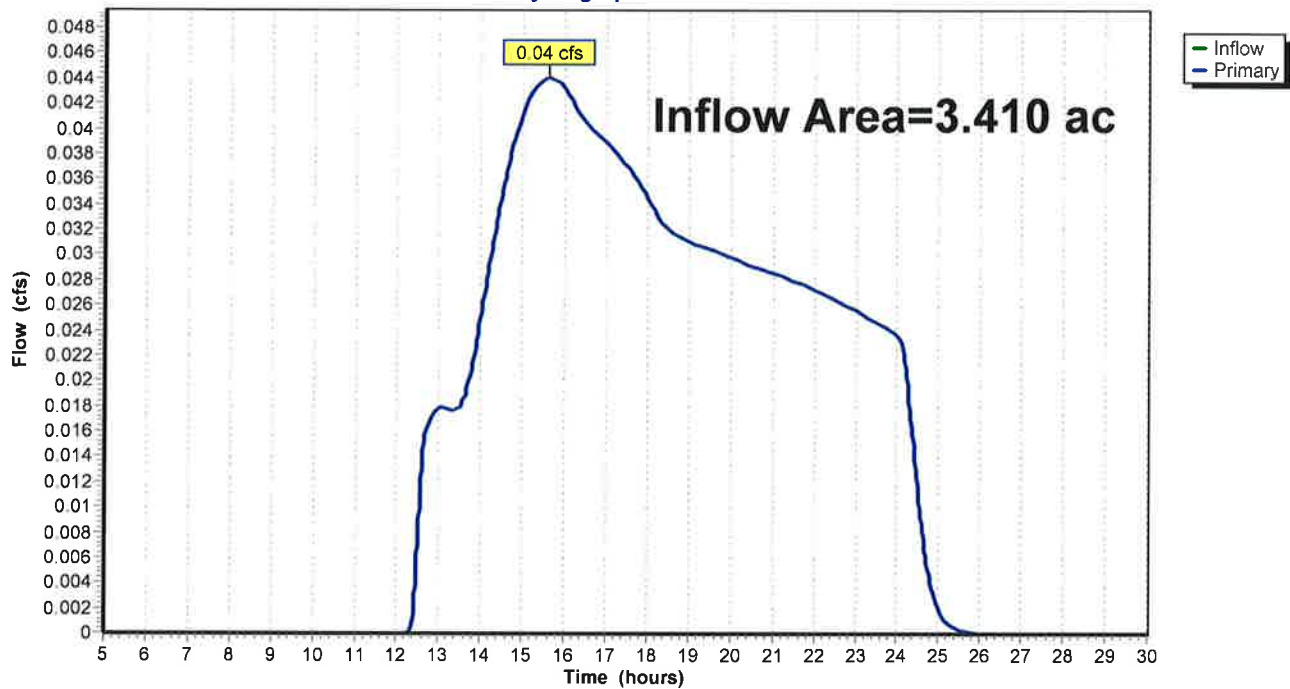
**Summary for Link 1L: Total Offsite**

Inflow Area = 3.410 ac, 0.88% Impervious, Inflow Depth = 0.11" for 25 Yr. Southern RI event  
Inflow = 0.04 cfs @ 15.63 hrs, Volume= 0.030 af  
Primary = 0.04 cfs @ 15.63 hrs, Volume= 0.030 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

Hydrograph



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Page 18

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 1.39 cfs @ 12.48 hrs, Volume= 0.268 af, Depth= 0.89"

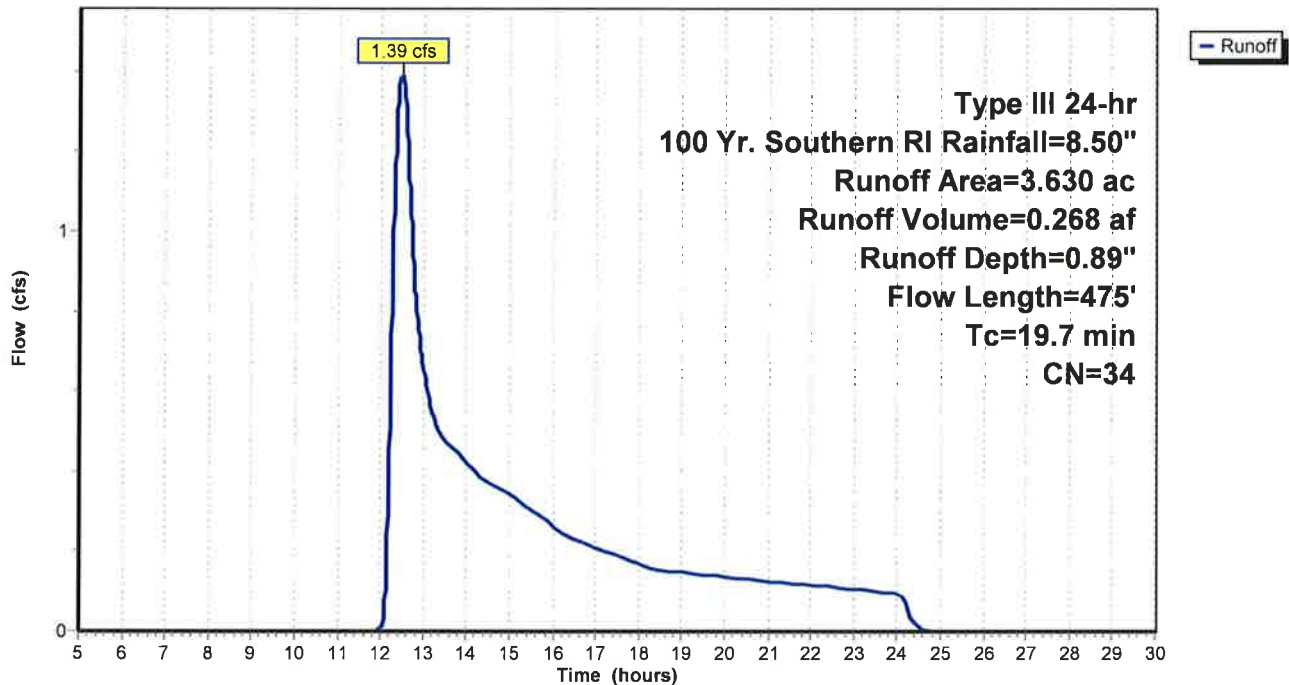
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

Area (ac)	CN	Description
2.880	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.630	34	Weighted Average
3.500		96.42% Pervious Area
0.130		3.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.8	115	0.1800	0.11		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	95	0.3800	3.08		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.5	125	0.0800	1.41		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
0.9	140	0.0280	2.69		<b>Shallow Concentrated Flow, Tt4</b> Unpaved Kv= 16.1 fps
19.7	475	Total			

### Subcatchment DA-1: Drainage Area 1

Hydrograph



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Pre-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

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Page 20

**Summary for Subcatchment DA-2: Drainage Area 2**

Runoff = 0.33 cfs @ 12.94 hrs, Volume= 0.128 af, Depth= 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

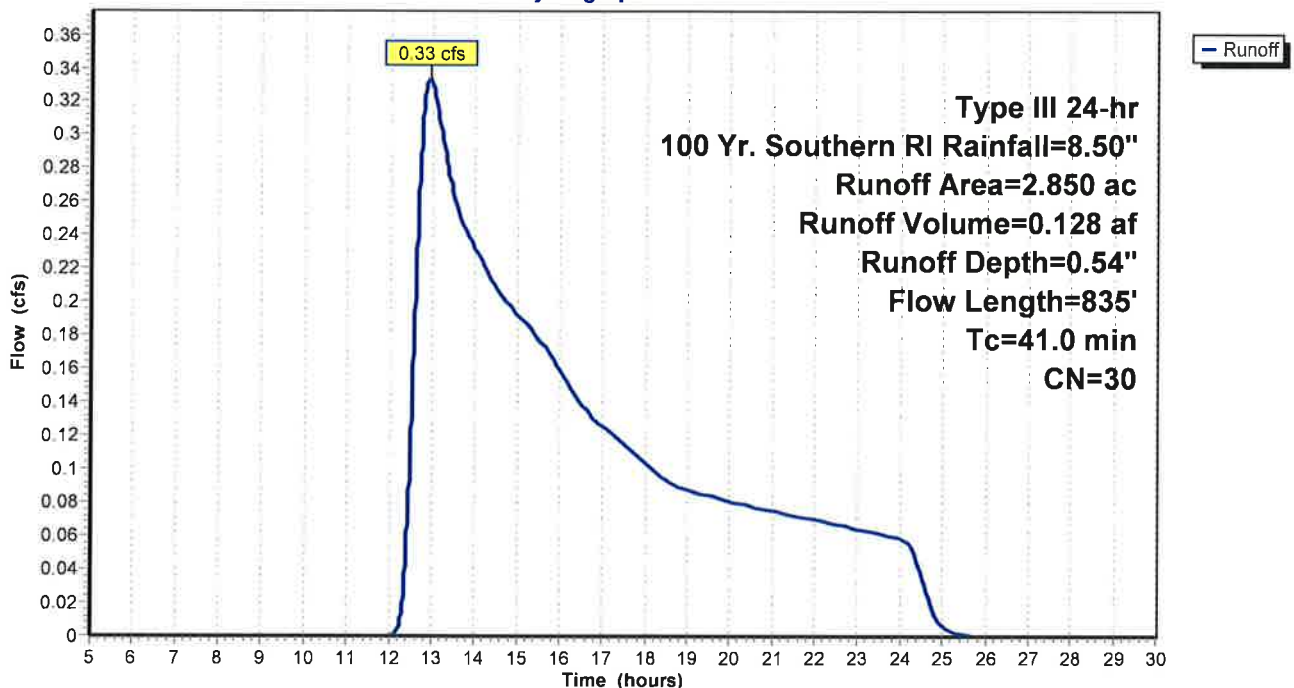
Area (ac)	CN	Description
2.850	30	Woods, Good, HSG A
2.850		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
25.5	155	0.1160	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.8	125	0.0480	0.55		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
6.8	345	0.0290	0.85		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
2.0	100	0.1100	0.83		<b>Shallow Concentrated Flow, Tt3</b> Forest w/Heavy Litter Kv= 2.5 fps
2.9	110	0.0640	0.63		<b>Shallow Concentrated Flow, Tt4</b> Forest w/Heavy Litter Kv= 2.5 fps
41.0	835	Total			

**Subcatchment DA-2: Drainage Area 2**

Hydrograph



**Hemmerle Trust-Pre Dev**

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Page 21

**Summary for Subcatchment DA-3: Drainage Area 3**

Runoff = 0.22 cfs @ 12.45 hrs, Volume= 0.041 af, Depth= 0.89"

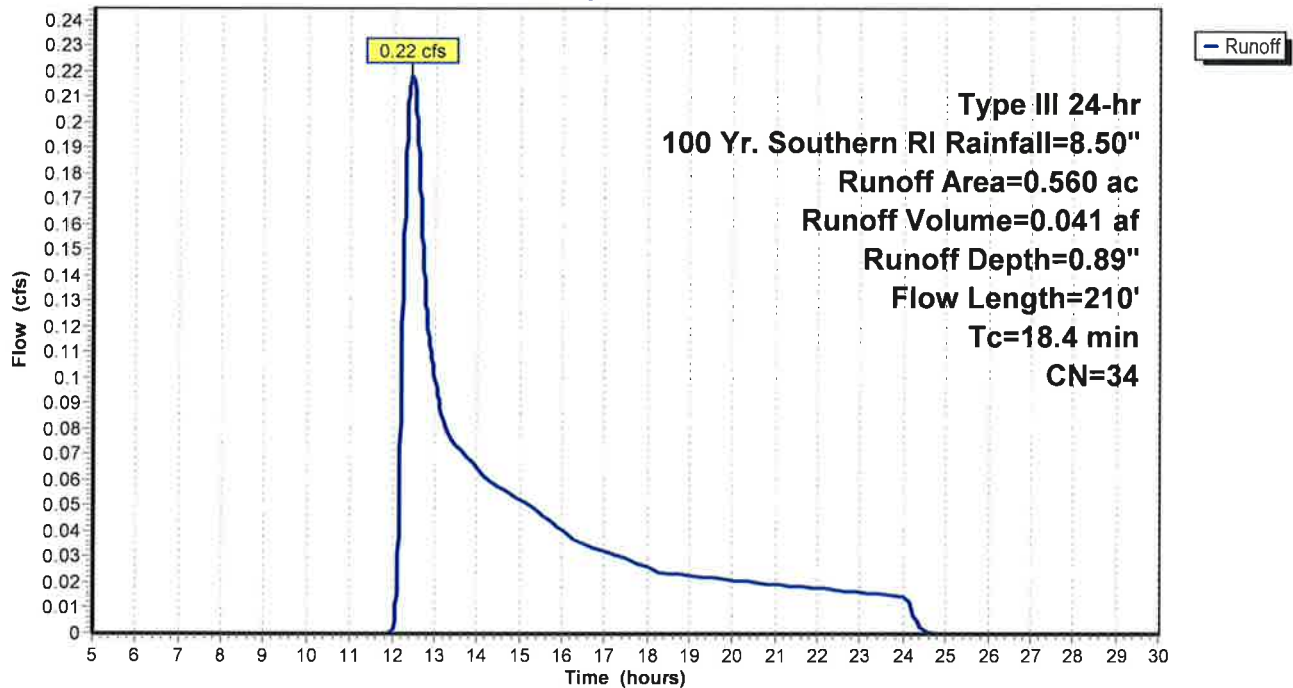
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

Area (ac)	CN	Description
0.510	30	Woods, Good, HSG A
0.030	98	Paved parking, HSG A
0.020	39	>75% Grass cover, Good, HSG A
0.560	34	Weighted Average
0.530		94.64% Pervious Area
0.030		5.36% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.1	110	0.0540	0.12		<b>Sheet Flow, Tt1</b> Woods: Light underbrush n= 0.400 P2= 3.40"
3.3	100	0.0100	0.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
18.4	210	Total			

**Subcatchment DA-3: Drainage Area 3**

Hydrograph



**Hemmerle Trust-Pre Dev**

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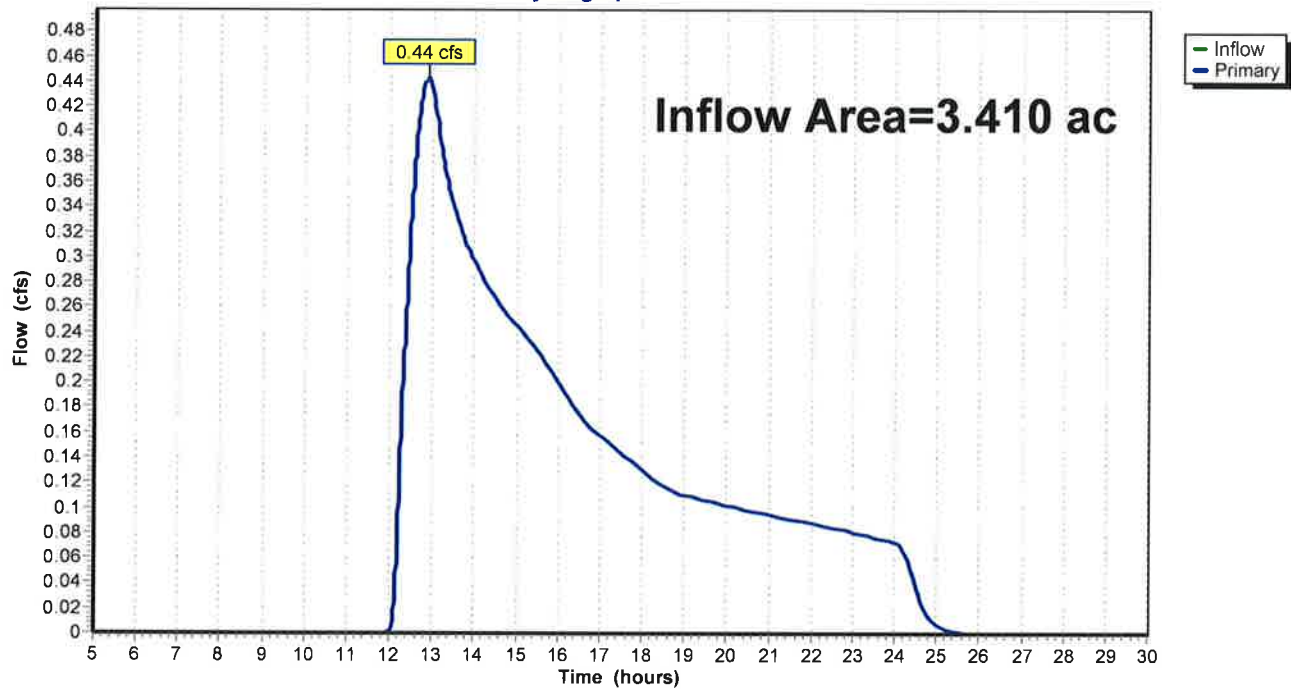
**Summary for Link 1L: Total Offsite**

Inflow Area = 3.410 ac, 0.88% Impervious, Inflow Depth = 0.60" for 100 Yr. Southern RI event  
Inflow = 0.44 cfs @ 12.89 hrs, Volume= 0.170 af  
Primary = 0.44 cfs @ 12.89 hrs, Volume= 0.170 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

Hydrograph



**ATTACHMENT 2**

**WATER QUALITY CALCULATIONS FOR SWALE  
AND  
GRASSED SWALE SIZING CALCULATIONS**

## WATER QUALITY CALCULATIONS FOR DRIVEWAY BioRetention Swale Along Road Edge

Note: The driveway is 850 feet long by 12 feet wide  
Each roadside swale will treat half of the road.

BMP: Bio-Retention Swale to Infiltration Basin

---

Impervious Area = 10,200.0 ft<sup>2</sup>                      0.23416 acres

WQV Rainfall Depth                      1.2 inches

F (A soils)                      0.6

F (B soils)                      0.35

F (C soils)                      0.25

F (D soils)                      0.1

WQV=                      850.0 ft<sup>3</sup>                      0.01951 ac.-ft.

Rev=                      510.0 ft<sup>3</sup>                      0.01171 ac.-ft.

### Sizing Bio-Retention Area

Minimum Filter Bed Surface Area

$$A_f = (WQv)(df) / [(k)(hf + df)(tf)]$$

df=                      0.25 ft.

k=                      1 ft/day

hf=                      0.183 ft.

tf=                      2 days

Af=                      245.4 s.f.

**Af (Prov)=                      1870 s.f.**                      (2.2 ft. wetted perimeter x 850 ft. swale length, 1,870 s.f.>245 s.f.)  
(Each swale capacity)

## Grassed Swale Receiving Runoff from DA-2A (Slope=2%)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	2.000 %
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Discharge	0.92 cfs
Results	
Normal Depth	3.4 in
Flow Area	0.5 ft <sup>2</sup>
Wetted Perimeter	2.8 ft
Hydraulic Radius	2.3 in
Top Width	2.72 ft
Critical Depth	2.8 in
Critical Slope	4.425 %
Velocity	1.73 ft/s
Velocity Head	0.05 ft
Specific Energy	0.33 ft
Froude Number	0.690
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	3.4 in
Critical Depth	2.8 in
Channel Slope	2.000 %
Critical Slope	4.425 %

## Grassed Swale Receiving Runoff from DA-2A (Slope=11%)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	11.000 %
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Discharge	0.92 cfs
Results	
Normal Depth	2.2 in
Flow Area	0.3 ft <sup>2</sup>
Wetted Perimeter	2.2 ft
Hydraulic Radius	1.6 in
Top Width	2.11 ft
Critical Depth	2.8 in
Critical Slope	4.424 %
Velocity	3.20 ft/s
Velocity Head	0.16 ft
Specific Energy	0.34 ft
Froude Number	1.530
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	2.2 in
Critical Depth	2.8 in
Channel Slope	11.000 %
Critical Slope	4.424 %

## Grassed Swale Receiving Runoff from DA-2B (Slope=2%)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	2.000 %
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Discharge	0.83 cfs
Results	
Normal Depth	3.3 in
Flow Area	0.5 ft <sup>2</sup>
Wetted Perimeter	2.7 ft
Hydraulic Radius	2.2 in
Top Width	2.63 ft
Critical Depth	2.7 in
Critical Slope	4.489 %
Velocity	1.68 ft/s
Velocity Head	0.04 ft
Specific Energy	0.32 ft
Froude Number	0.685
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	3.3 in
Critical Depth	2.7 in
Channel Slope	2.000 %
Critical Slope	4.489 %

## Grassed Swale Receiving Runoff from DA-2B (Slope=11%)

Project Description	
Friction Method	Manning
	Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.040
Channel Slope	11.000 %
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Discharge	0.83 cfs
Results	
Normal Depth	2.1 in
Flow Area	0.3 ft <sup>2</sup>
Wetted Perimeter	2.1 ft
Hydraulic Radius	1.5 in
Top Width	2.05 ft
Critical Depth	2.7 in
Critical Slope	4.490 %
Velocity	3.11 ft/s
Velocity Head	0.15 ft
Specific Energy	0.33 ft
Froude Number	1.518
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	2.1 in
Critical Depth	2.7 in
Channel Slope	11.000 %
Critical Slope	4.490 %

## Worksheet for Hemmerle Trust-Grassed Swale at 2.0% Slope

Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.040
Channel Slope	2.000 %
Normal Depth	12.0 in
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Results	
Discharge	14.04 cfs ← <b>MAX. DISCHARGE CAPACITY OF SWALE</b>
Flow Area	4.0 ft <sup>2</sup>
Wetted Perimeter	7.3 ft
Hydraulic Radius	6.6 in
Top Width	7.00 ft
Critical Depth	11.0 in
Critical Slope	3.064 %
Velocity	3.51 ft/s ← <b>MAX VELOCITY</b>
Velocity Head	0.19 ft
Specific Energy	1.19 ft
Froude Number	0.819
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	12.0 in
Critical Depth	11.0 in
Channel Slope	2.000 %
Critical Slope	3.064 %

## Worksheet for Hemmerle Trust-Grassed Swale at 2.6% Slope

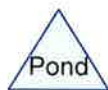
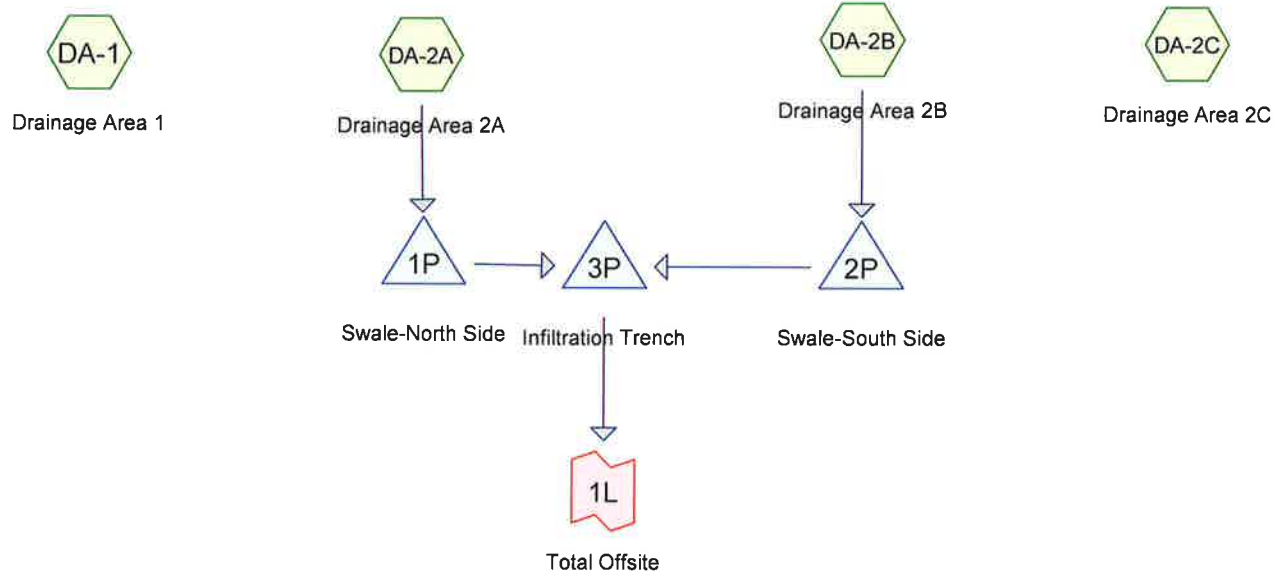
Project Description	
Friction Method	Manning
	Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.040
Channel Slope	2.600 %
Normal Depth	12.0 in
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Results	
Discharge	16.01 cfs ← MAX DISCHARGE
Flow Area	4.0 ft <sup>2</sup> ← CAPACITY OF SWALE
Wetted Perimeter	7.3 ft
Hydraulic Radius	6.6 in
Top Width	7.00 ft
Critical Depth	11.6 in
Critical Slope	3.012 %
Velocity	4.00 ft/s ← MAX VELOCITY
Velocity Head	0.25 ft
Specific Energy	1.25 ft
Froude Number	0.933
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	12.0 in
Critical Depth	11.6 in
Channel Slope	2.600 %
Critical Slope	3.012 %

## Worksheet for Hemmerle Trust-Grassed Swale at 11% Slope

Project Description	
Friction Method	Manning Formula
Solve For	Discharge
Input Data	
Roughness Coefficient	0.040
Channel Slope	11.000 %
Normal Depth	12.0 in
Left Side Slope	3.000 H:V
Right Side Slope	3.000 H:V
Bottom Width	1.00 ft
Results	
Discharge	32.93 cfs ← MAX. DISCHARGE CAPACITY OF SWALE
Flow Area	4.0 ft <sup>2</sup>
Wetted Perimeter	7.3 ft
Hydraulic Radius	6.6 in
Top Width	7.00 ft
Critical Depth	16.1 in
Critical Slope	2.739 %
Velocity	8.23 ft/s ← MAX. VELOCITY
Velocity Head	1.05 ft
Specific Energy	2.05 ft
Froude Number	1.920
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	12.0 in
Critical Depth	16.1 in
Channel Slope	11.000 %
Critical Slope	2.739 %

**ATTACHMENT 3**

**POST DEVELOPMENT DRAINAGE CALCULATIONS**



**Hemmerle Trust-Post Dev**

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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

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Page 2

**Summary for Subcatchment DA-1: Drainage Area 1**

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

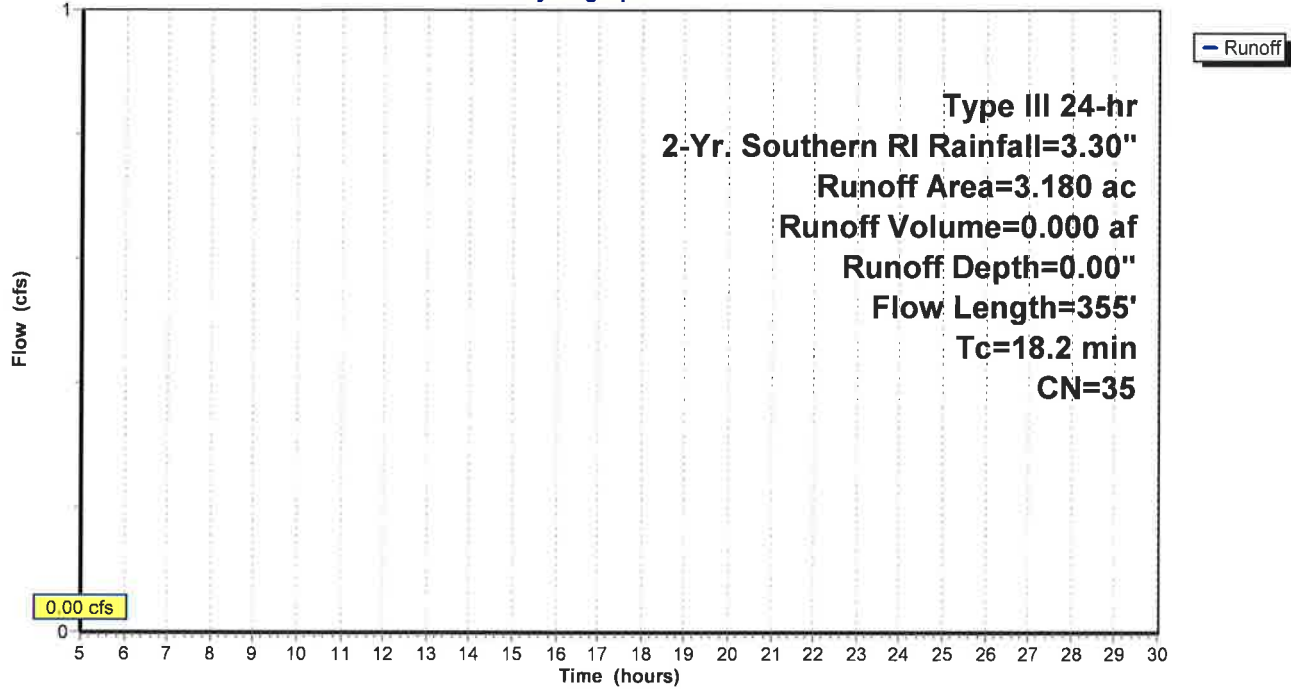
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

Area (ac)	CN	Description
2.430	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.180	35	Weighted Average
3.050		95.91% Pervious Area
0.130		4.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8	150	0.3600	0.16		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	80	0.2500	2.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.9	125	0.0480	1.10		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
18.2	355	Total			

### Subcatchment DA-1: Drainage Area 1

Hydrograph



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Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

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Page 4

**Summary for Subcatchment DA-2A: Drainage Area 2A**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

---

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

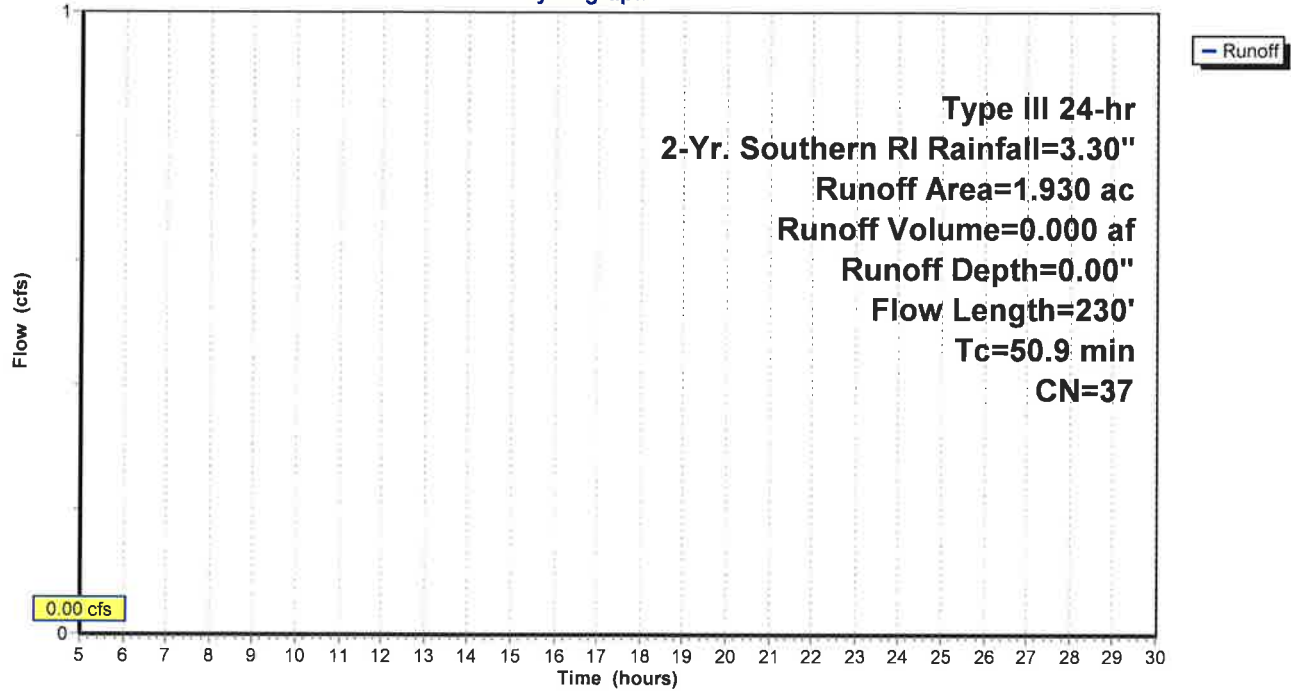
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

Area (ac)	CN	Description
1.200	30	Woods, Good, HSG A
0.610	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
1.930	37	Weighted Average
1.810		93.78% Pervious Area
0.120		6.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	120	0.0330	0.06		<b>Sheet Flow, Tt1</b>
					Woods: Dense underbrush n= 0.800 P2= 3.40"
1.6	110	0.0270	1.15		<b>Shallow Concentrated Flow, Tt2</b>
					Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
50.9	230	Total			

### Subcatchment DA-2A: Drainage Area 2A

Hydrograph





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Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

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Page 7

**Summary for Subcatchment DA-2C: Drainage Area 2C**

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

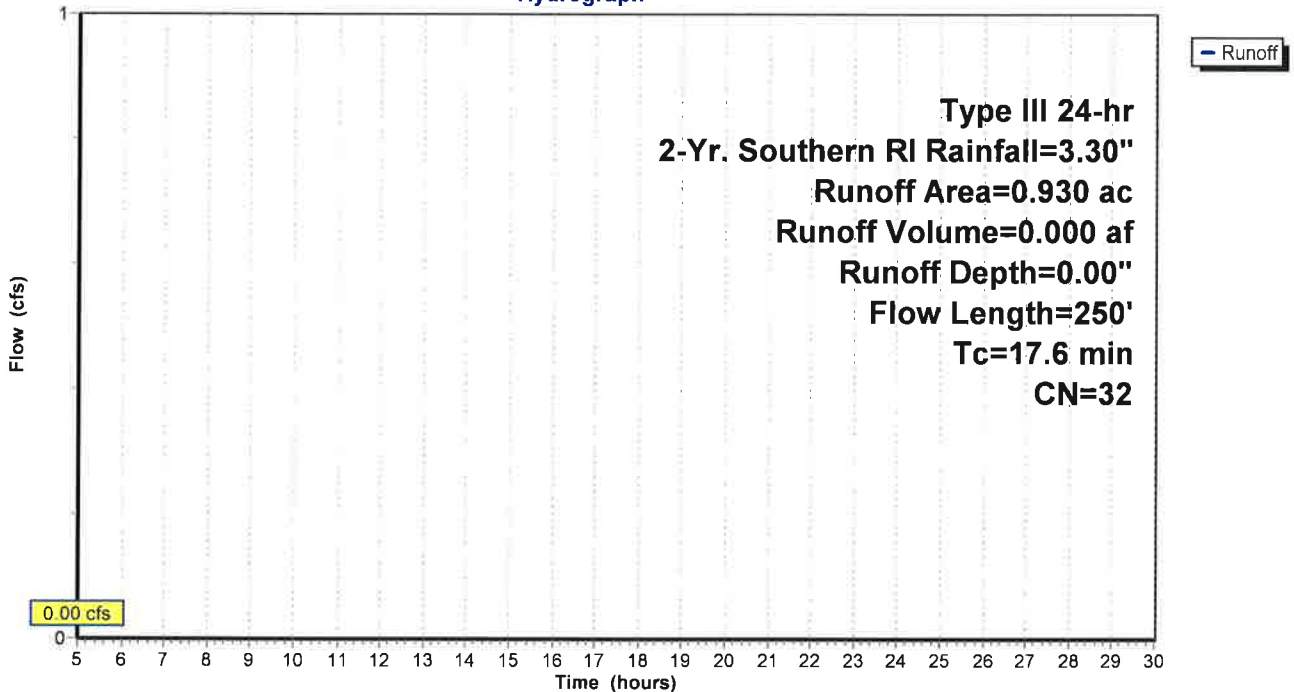
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

Area (ac)	CN	Description
0.700	30	Woods, Good, HSG A
0.230	39	>75% Grass cover, Good, HSG A
0.930	32	Weighted Average
0.930		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.2200	0.12		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.7	150	0.0730	0.68		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
17.6	250	Total			

**Subcatchment DA-2C: Drainage Area 2C**

Hydrograph



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Page 8

**Summary for Pond 1P: Swale-North Side**

Inflow Area = 1.930 ac, 6.22% Impervious, Inflow Depth = 0.00" for 2-Yr. Southern RI event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 102.70' @ 5.00 hrs Surf.Area= 200 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)  
 Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

Device	Routing	Invert	Outlet Devices															
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'															
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32															

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' (Free Discharge)

↑1=Exfiltration (Passes 0.00 cfs of 0.04 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' TW=96.75' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

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Post-Development Drainage Analysis-350 Ministerial Rd

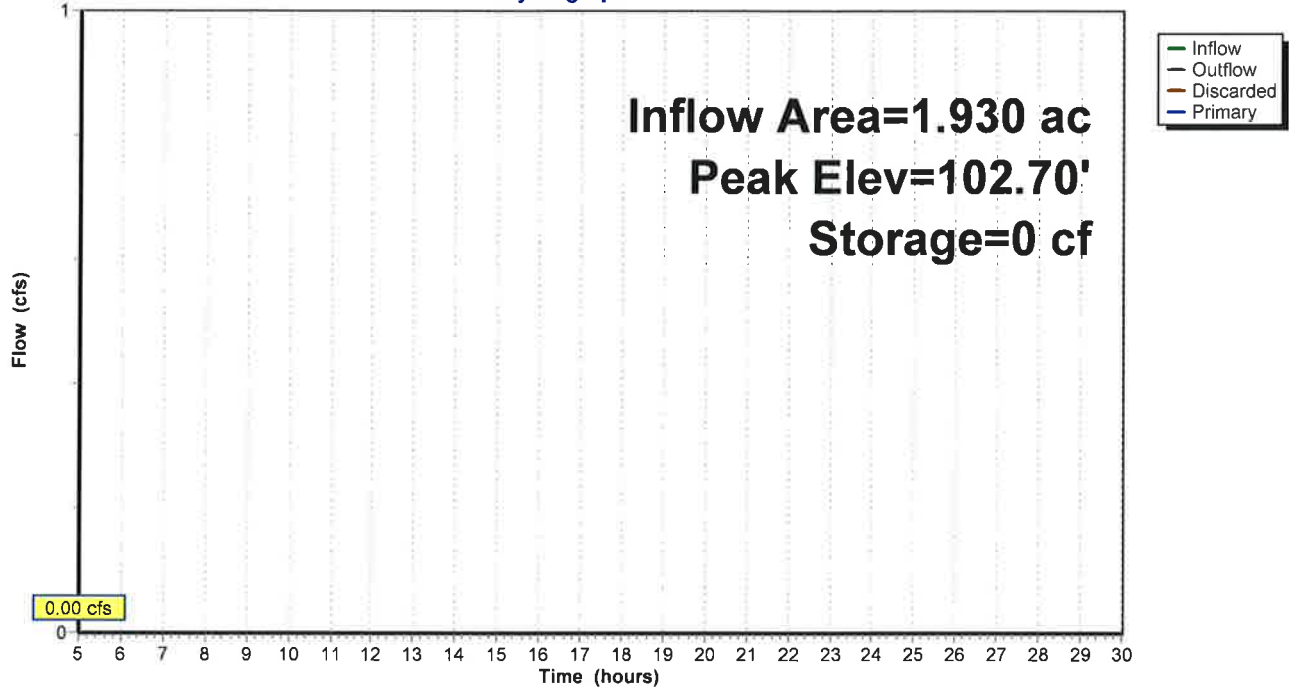
Type III 24-hr 2-Yr. Southern RI Rainfall=3.30"

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Page 9

**Pond 1P: Swale-North Side**

Hydrograph



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Page 10

**Summary for Pond 2P: Swale-South Side**

Inflow Area = 0.930 ac, 12.90% Impervious, Inflow Depth = 0.01" for 2-Yr. Southern RI event  
 Inflow = 0.00 cfs @ 22.30 hrs, Volume= 0.001 af  
 Outflow = 0.00 cfs @ 22.30 hrs, Volume= 0.001 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.00 cfs @ 22.30 hrs, Volume= 0.001 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 102.70' @ 5.00 hrs Surf.Area= 200 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.0 min ( 1,252.3 - 1,252.3 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

Device	Routing	Invert	Outlet Devices											
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'											
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32											

**Discarded OutFlow** Max=0.00 cfs @ 22.30 hrs HW=102.70' (Free Discharge)

↑1=Exfiltration (Passes 0.00 cfs of 0.04 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' TW=96.75' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)



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Page 12

**Summary for Pond 3P: Infiltration Trench**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 2-Yr. Southern RI event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 96.75' @ 5.00 hrs Surf.Area= 108 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)  
 Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	96.75'	133 cf	<b>6.00'W x 18.00'L x 5.25'H Prismatic</b> 567 cf Overall - 236 cf Embedded = 331 cf x 40.0% Voids
#2	97.75'	177 cf	<b>Concrete Galley 4x4x4 x 4 Inside #1</b> Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf
#3	102.00'	1,728 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>
		2,038 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
102.00	300	0	0	300
102.50	385	171	171	391
103.00	520	225	396	531
103.35	620	199	595	636
104.00	3,200	1,133	1,728	3,217

Device	Routing	Invert	Outlet Devices
#1	Discarded	96.75'	<b>12.000 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'
#2	Primary	103.50'	<b>5.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=96.75' (Free Discharge)

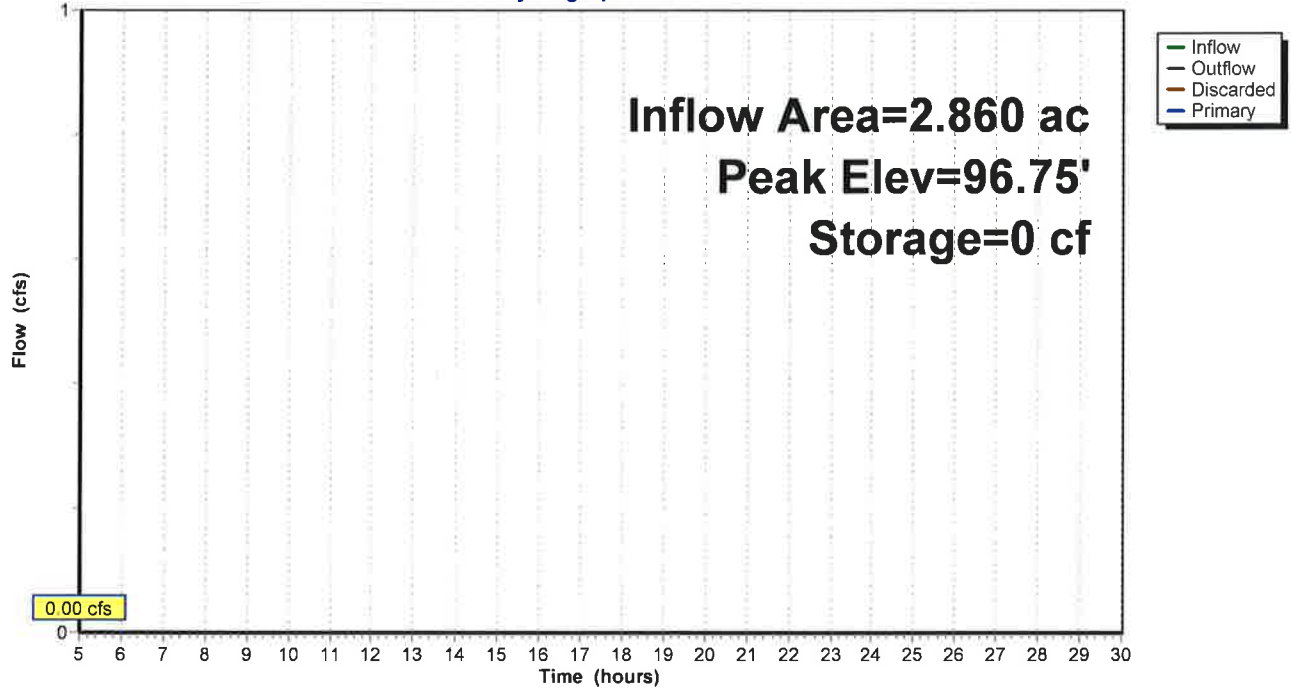
↑1=Exfiltration (Passes 0.00 cfs of 0.03 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=96.75' TW=0.00' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 3P: Infiltration Trench

Hydrograph



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Page 14

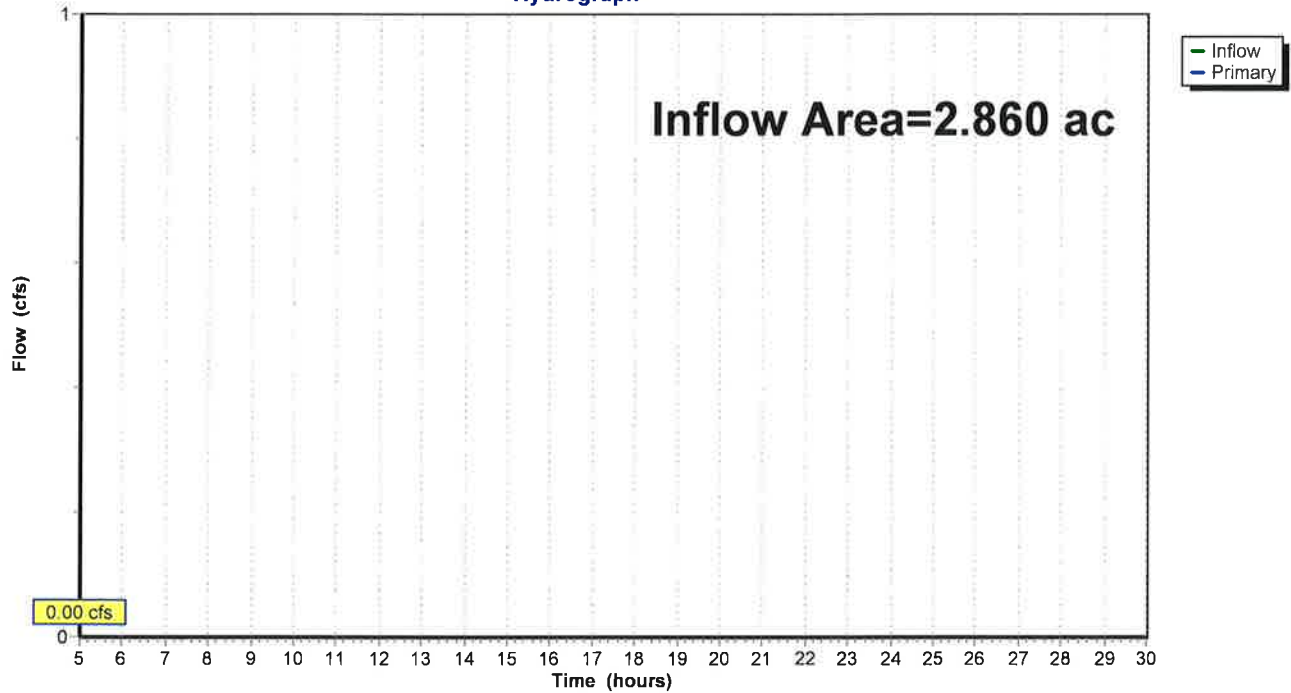
**Summary for Link 1L: Total Offsite**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 2-Yr. Southern RI event  
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

**Hydrograph**



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Page 15

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 0.03 cfs @ 15.47 hrs, Volume= 0.019 af, Depth= 0.07"

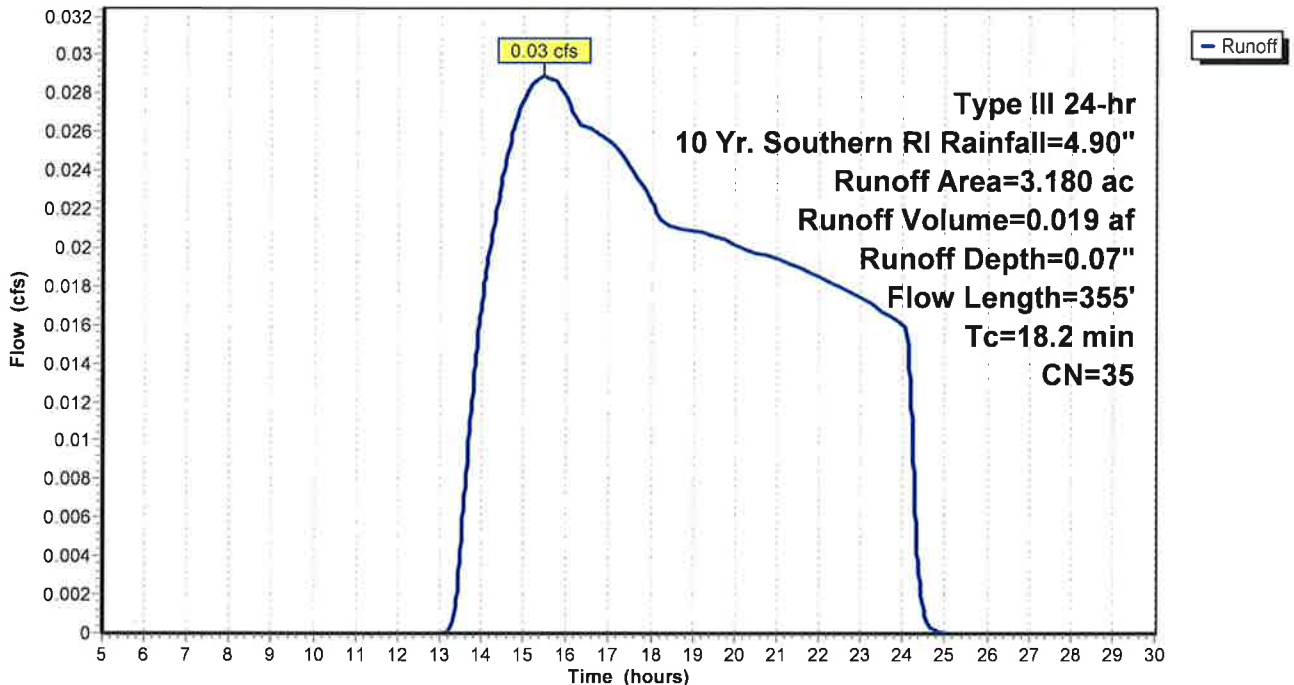
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

Area (ac)	CN	Description
2.430	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.180	35	Weighted Average
3.050		95.91% Pervious Area
0.130		4.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8	150	0.3600	0.16		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	80	0.2500	2.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.9	125	0.0480	1.10		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
18.2	355	Total			

**Subcatchment DA-1: Drainage Area 1**

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

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Page 16

**Summary for Subcatchment DA-2A: Drainage Area 2A**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.03 cfs @ 15.44 hrs, Volume= 0.019 af, Depth= 0.12"

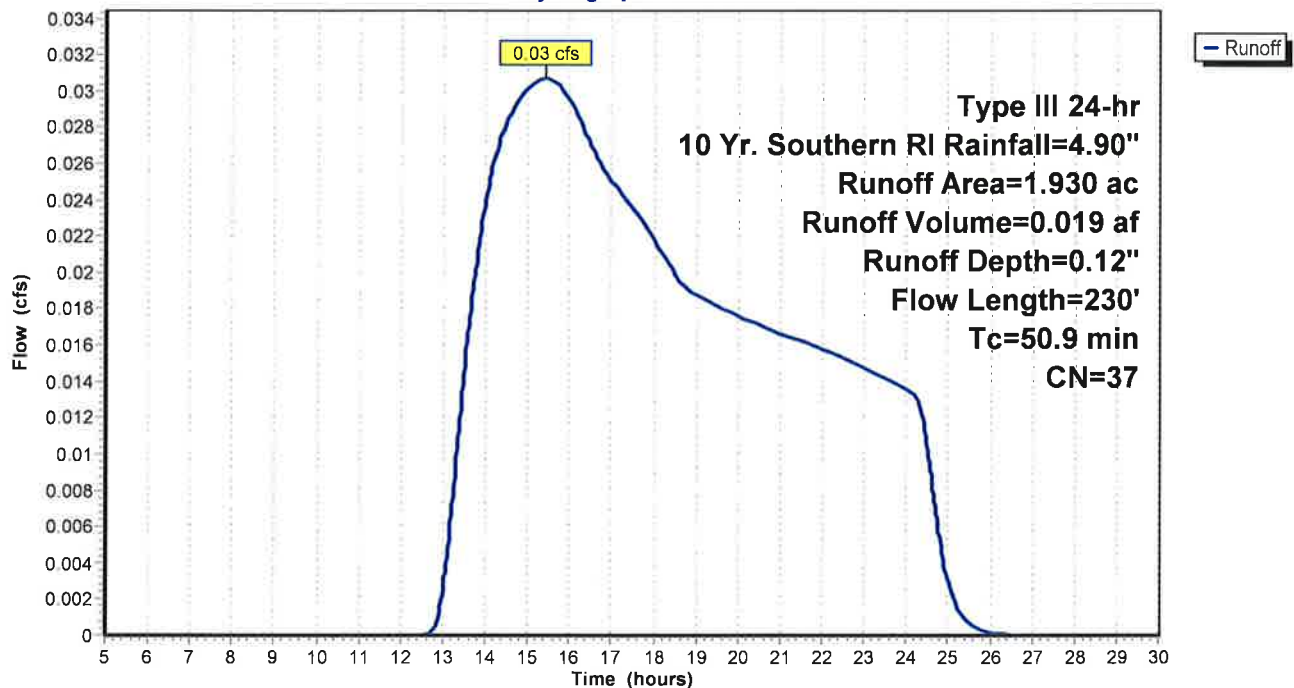
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

Area (ac)	CN	Description
1.200	30	Woods, Good, HSG A
0.610	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
1.930	37	Weighted Average
1.810		93.78% Pervious Area
0.120		6.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	120	0.0330	0.06		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
1.6	110	0.0270	1.15		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
50.9	230	Total			

**Subcatchment DA-2A: Drainage Area 2A**

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

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Page 17

**Summary for Subcatchment DA-2B: Drainage Area 2B**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.04 cfs @ 12.98 hrs, Volume= 0.019 af, Depth= 0.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

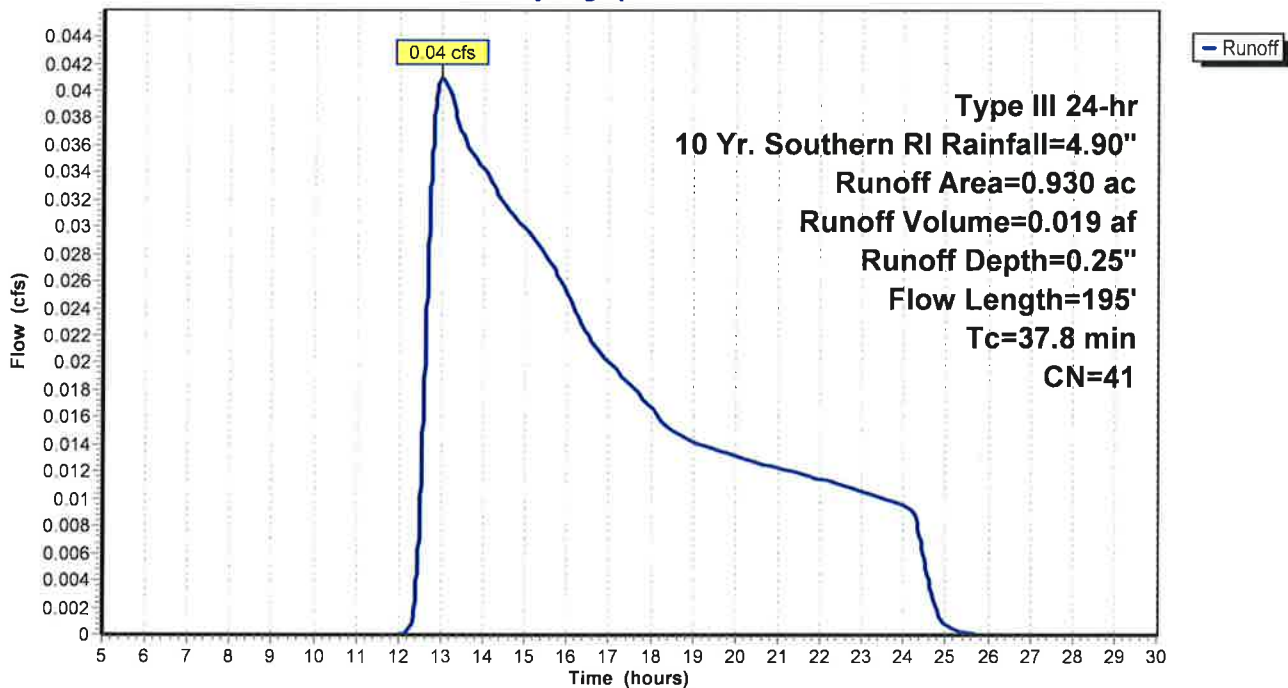
Area (ac)	CN	Description
0.600	30	Woods, Good, HSG A
0.210	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
0.930	41	Weighted Average
0.810		87.10% Pervious Area
0.120		12.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.2	130	0.1150	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.6	65	0.0600	1.71		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
37.8	195	Total			

**Subcatchment DA-2B: Drainage Area 2B**

Hydrograph



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Page 18

**Summary for Subcatchment DA-2C: Drainage Area 2C**

Runoff = 0.00 cfs @ 21.88 hrs, Volume= 0.001 af, Depth= 0.02"

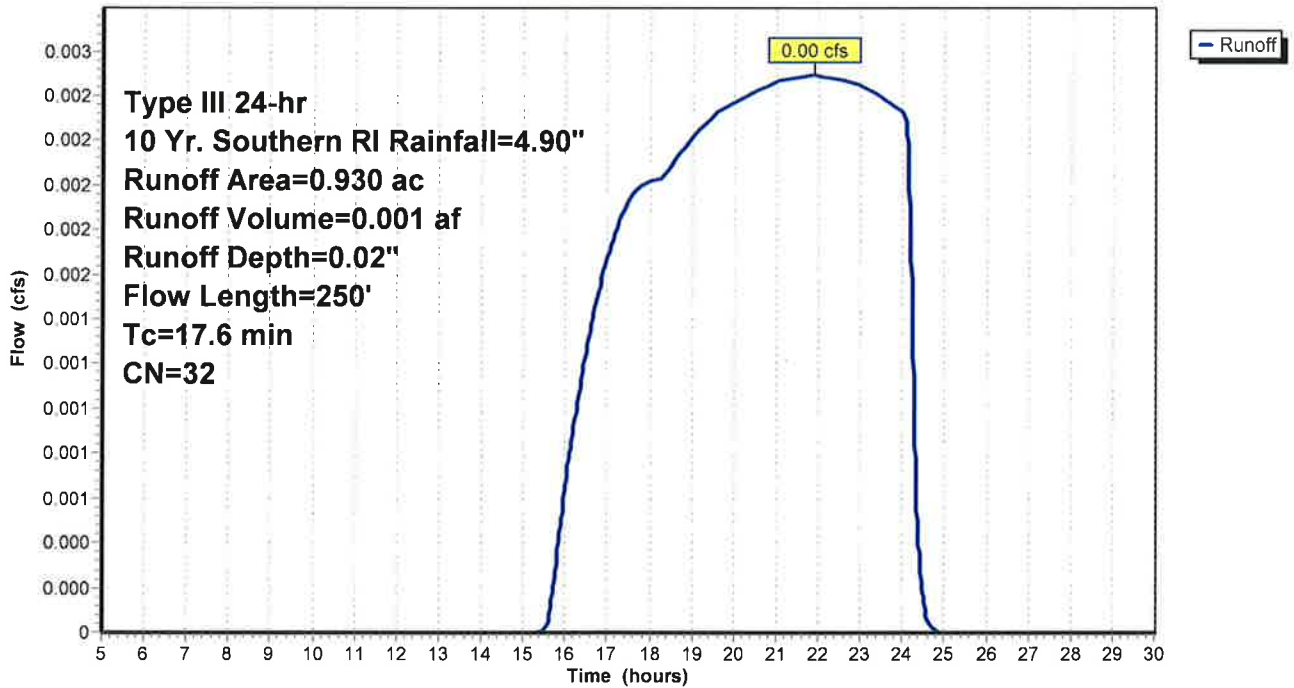
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

Area (ac)	CN	Description
0.700	30	Woods, Good, HSG A
0.230	39	>75% Grass cover, Good, HSG A
0.930	32	Weighted Average
0.930		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.2200	0.12		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.7	150	0.0730	0.68		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
17.6	250	Total			

**Subcatchment DA-2C: Drainage Area 2C**

Hydrograph



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Page 19

**Summary for Pond 1P: Swale-North Side**

Inflow Area = 1.930 ac, 6.22% Impervious, Inflow Depth = 0.12" for 10 Yr. Southern RI event  
 Inflow = 0.03 cfs @ 15.44 hrs, Volume= 0.019 af  
 Outflow = 0.03 cfs @ 15.44 hrs, Volume= 0.019 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.03 cfs @ 15.44 hrs, Volume= 0.019 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 102.70' @ 15.44 hrs Surf.Area= 200 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.0 min ( 1,095.3 - 1,095.3 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

Device	Routing	Invert	Outlet Devices															
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'															
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32															

**Discarded OutFlow** Max=0.04 cfs @ 15.44 hrs HW=102.70' (Free Discharge)

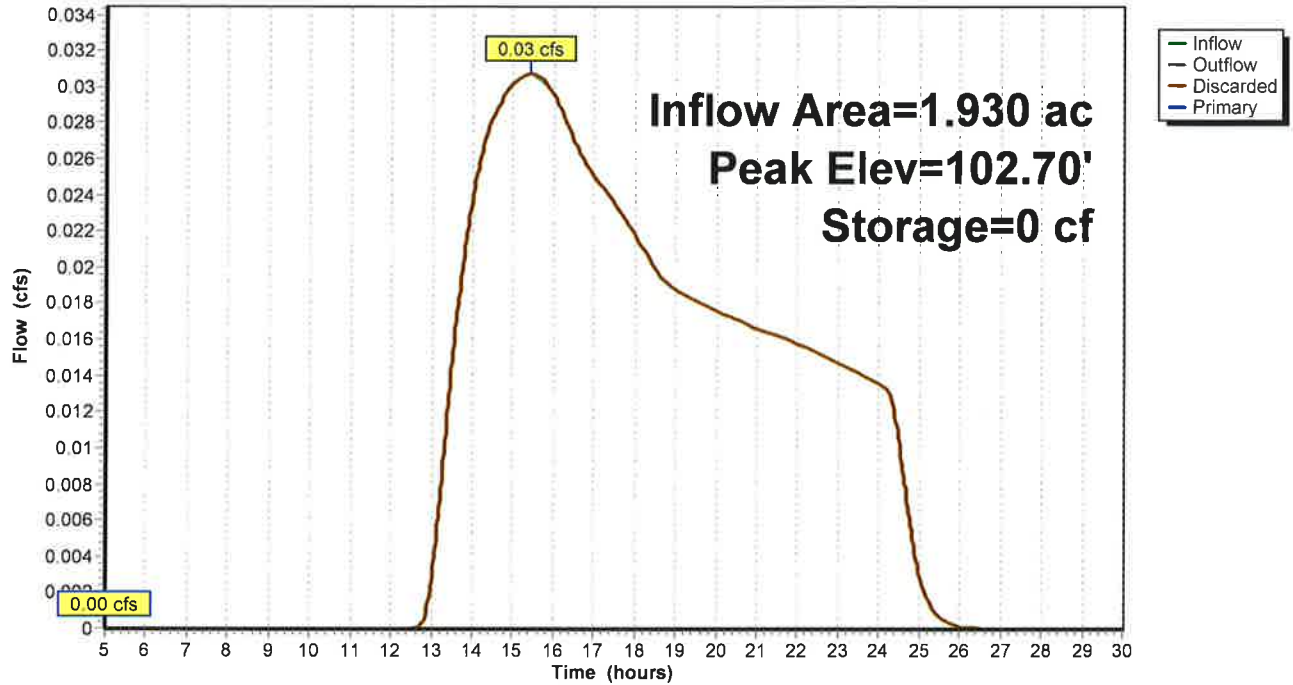
↑1=Exfiltration ( Controls 0.04 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' TW=96.75' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 1P: Swale-North Side

Hydrograph



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Page 21

**Summary for Pond 2P: Swale-South Side**

Inflow Area = 0.930 ac, 12.90% Impervious, Inflow Depth = 0.25" for 10 Yr. Southern RI event  
 Inflow = 0.04 cfs @ 12.98 hrs, Volume= 0.019 af  
 Outflow = 0.04 cfs @ 13.25 hrs, Volume= 0.019 af, Atten= 4%, Lag= 16.3 min  
 Discarded = 0.04 cfs @ 13.25 hrs, Volume= 0.019 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 102.71' @ 13.25 hrs Surf.Area= 204 sf Storage= 2 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.1 min ( 1,019.5 - 1,019.4 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

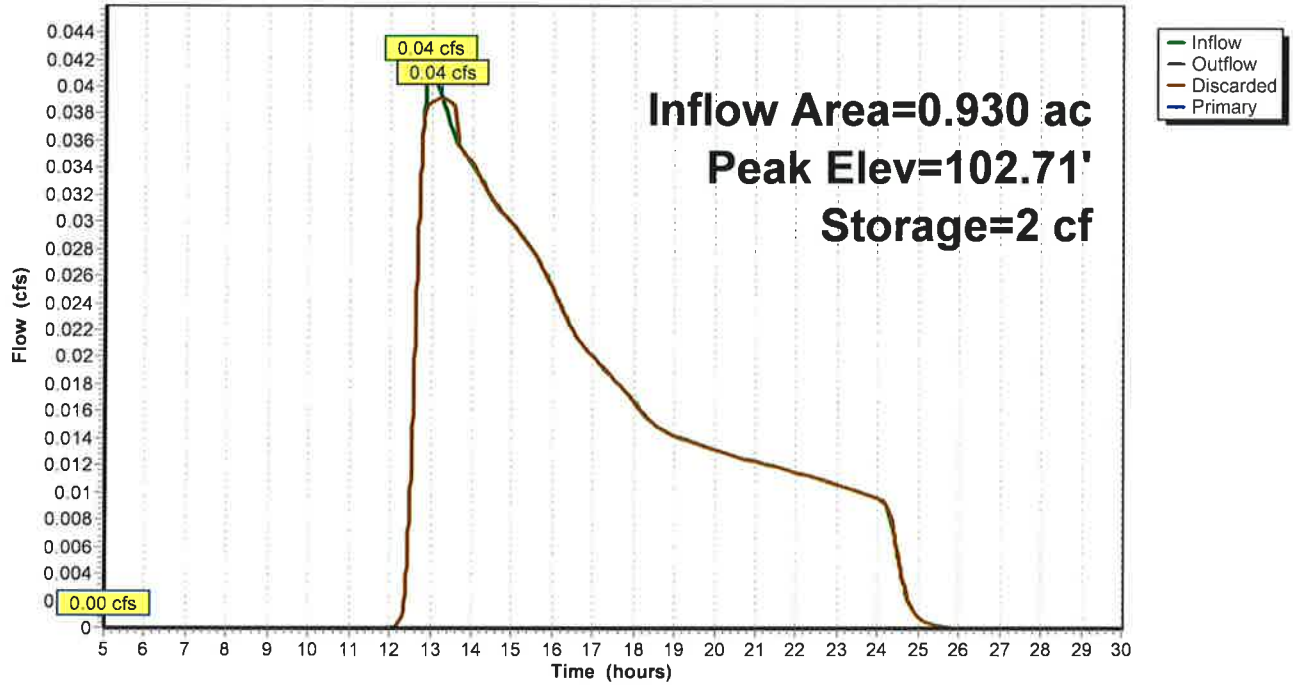
Device	Routing	Invert	Outlet Devices											
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'											
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32											

**Discarded OutFlow** Max=0.04 cfs @ 13.25 hrs HW=102.71' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.04 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' TW=96.75' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

**Pond 2P: Swale-South Side**

Hydrograph



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Page 23

**Summary for Pond 3P: Infiltration Trench**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 10 Yr. Southern RI event  
 Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 96.75' @ 5.00 hrs Surf.Area= 108 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)  
 Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	96.75'	133 cf	<b>6.00'W x 18.00'L x 5.25'H Prismatic</b> 567 cf Overall - 236 cf Embedded = 331 cf x 40.0% Voids
#2	97.75'	177 cf	<b>Concrete Galley 4x4x4 x 4 Inside #1</b> Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf
#3	102.00'	1,728 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>
		2,038 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
102.00	300	0	0	300
102.50	385	171	171	391
103.00	520	225	396	531
103.35	620	199	595	636
104.00	3,200	1,133	1,728	3,217

Device	Routing	Invert	Outlet Devices
#1	Discarded	96.75'	<b>12.000 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'
#2	Primary	103.50'	<b>5.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Discarded OutFlow** Max=0.00 cfs @ 5.00 hrs HW=96.75' (Free Discharge)

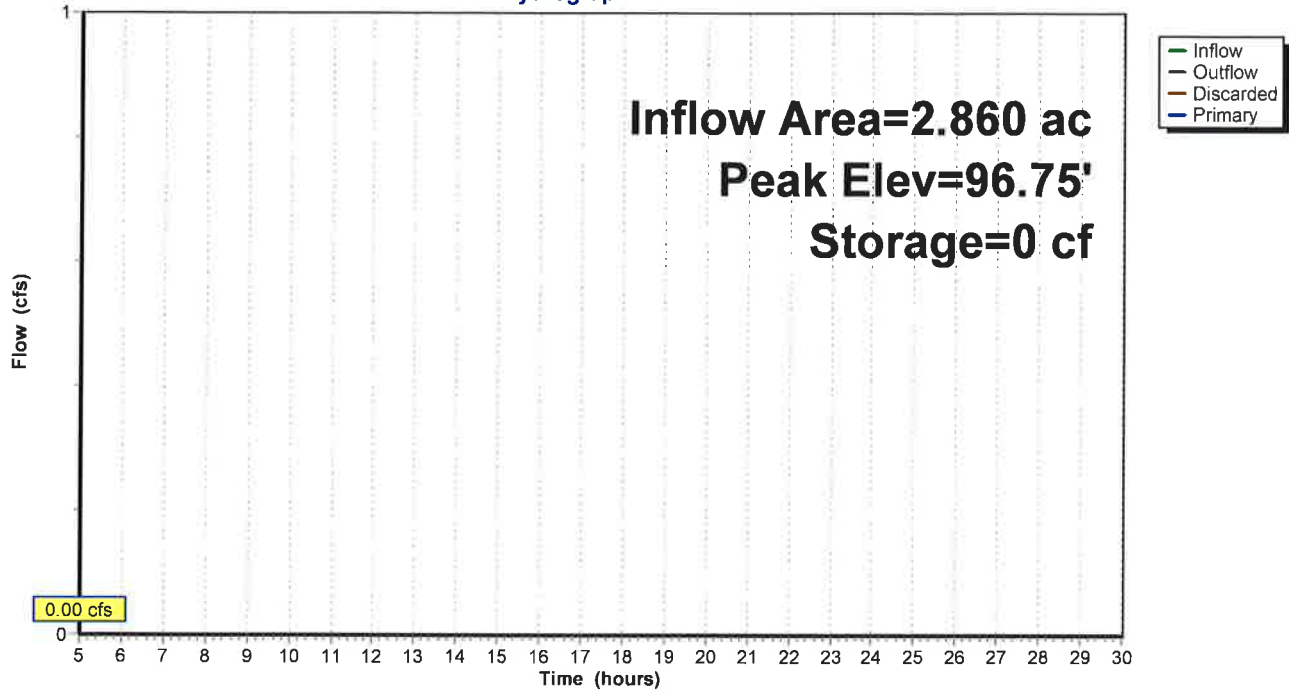
↑1=Exfiltration (Passes 0.00 cfs of 0.03 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=96.75' TW=0.00' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 3P: Infiltration Trench

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 10 Yr. Southern RI Rainfall=4.90"

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Page 25

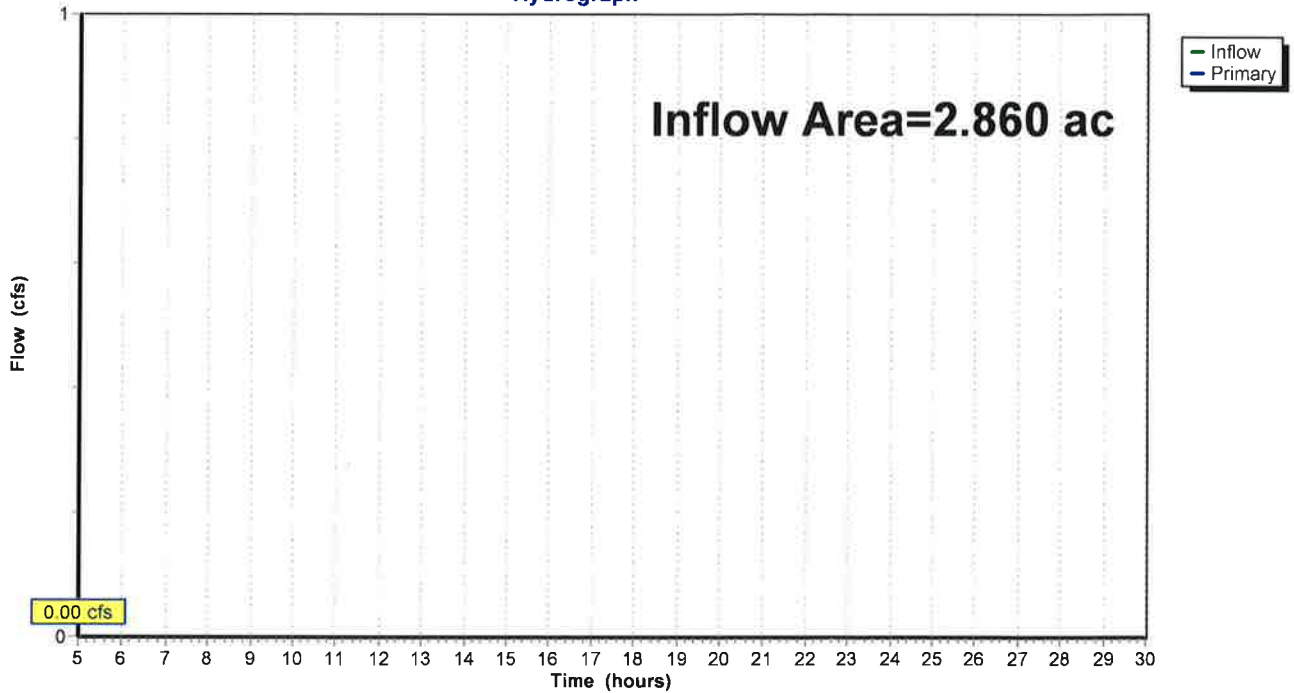
**Summary for Link 1L: Total Offsite**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 10 Yr. Southern RI event  
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

Hydrograph



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Page 26

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 0.16 cfs @ 12.64 hrs, Volume= 0.072 af, Depth= 0.27"

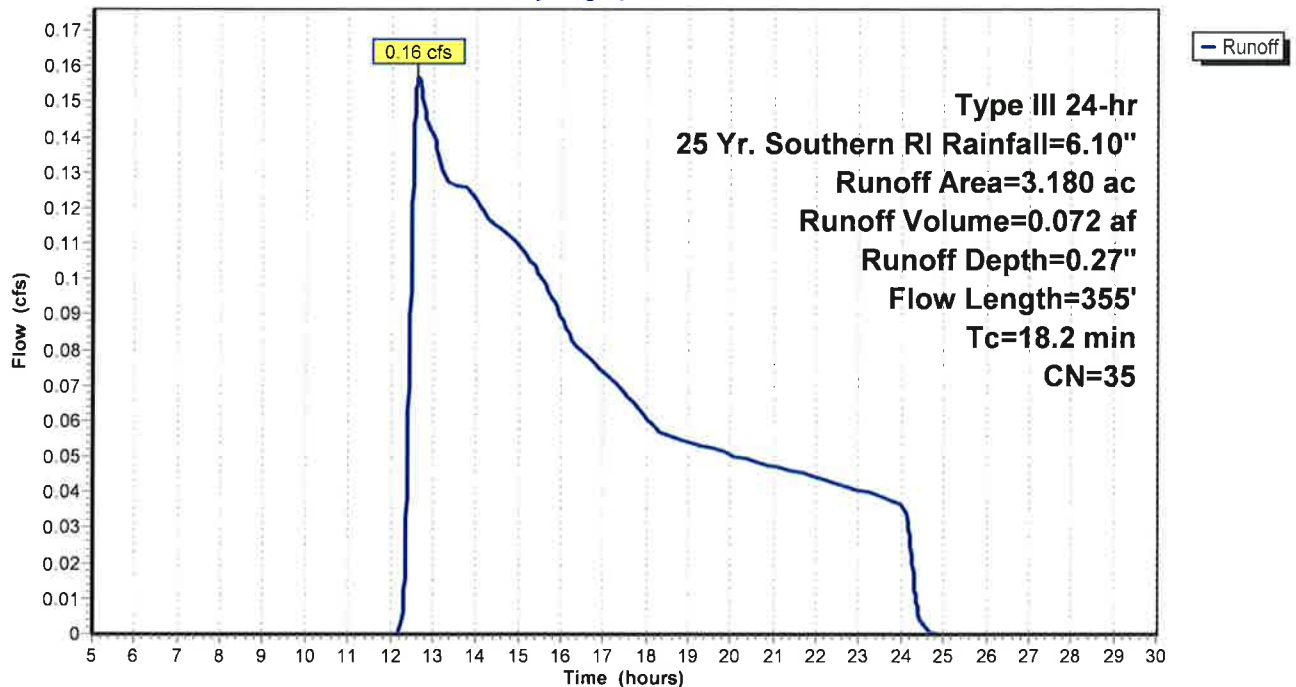
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

Area (ac)	CN	Description
2.430	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.180	35	Weighted Average
3.050		95.91% Pervious Area
0.130		4.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8	150	0.3600	0.16		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	80	0.2500	2.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.9	125	0.0480	1.10		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
18.2	355	Total			

**Subcatchment DA-1: Drainage Area 1**

Hydrograph



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Page 27

**Summary for Subcatchment DA-2A: Drainage Area 2A**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.14 cfs @ 13.17 hrs, Volume= 0.059 af, Depth= 0.37"

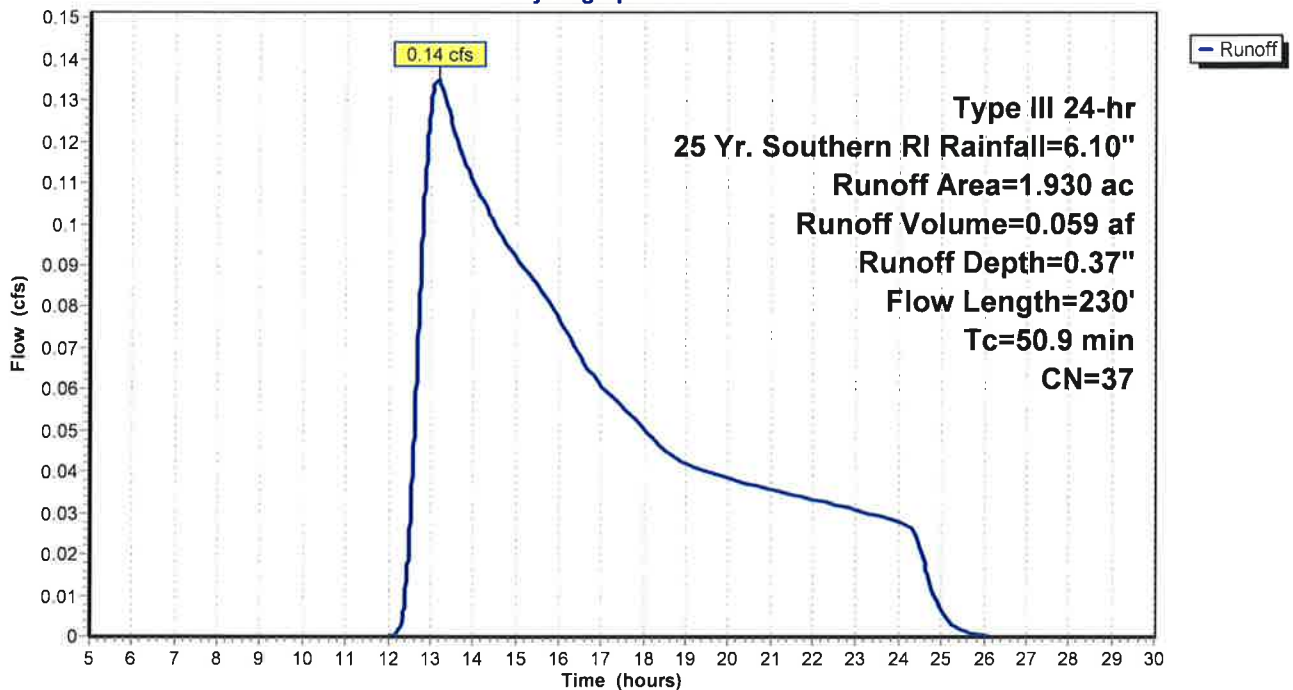
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

Area (ac)	CN	Description
1.200	30	Woods, Good, HSG A
0.610	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
1.930	37	Weighted Average
1.810		93.78% Pervious Area
0.120		6.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	120	0.0330	0.06		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
1.6	110	0.0270	1.15		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
50.9	230	Total			

**Subcatchment DA-2A: Drainage Area 2A**

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

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Page 28

**Summary for Subcatchment DA-2B: Drainage Area 2B**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.18 cfs @ 12.73 hrs, Volume= 0.046 af, Depth= 0.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

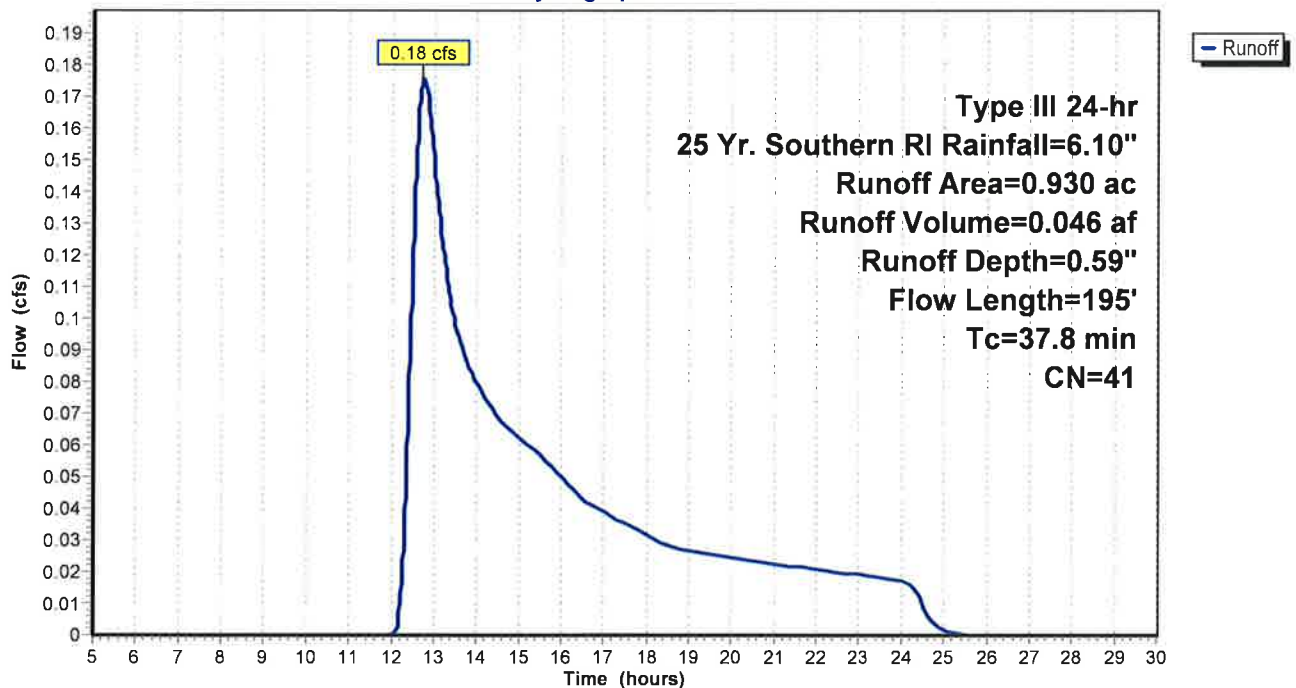
Area (ac)	CN	Description
0.600	30	Woods, Good, HSG A
0.210	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
0.930	41	Weighted Average
0.810		87.10% Pervious Area
0.120		12.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.2	130	0.1150	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.6	65	0.0600	1.71		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
37.8	195	Total			

**Subcatchment DA-2B: Drainage Area 2B**

Hydrograph



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Page 29

**Summary for Subcatchment DA-2C: Drainage Area 2C**

Runoff = 0.02 cfs @ 14.92 hrs, Volume= 0.011 af, Depth= 0.15"

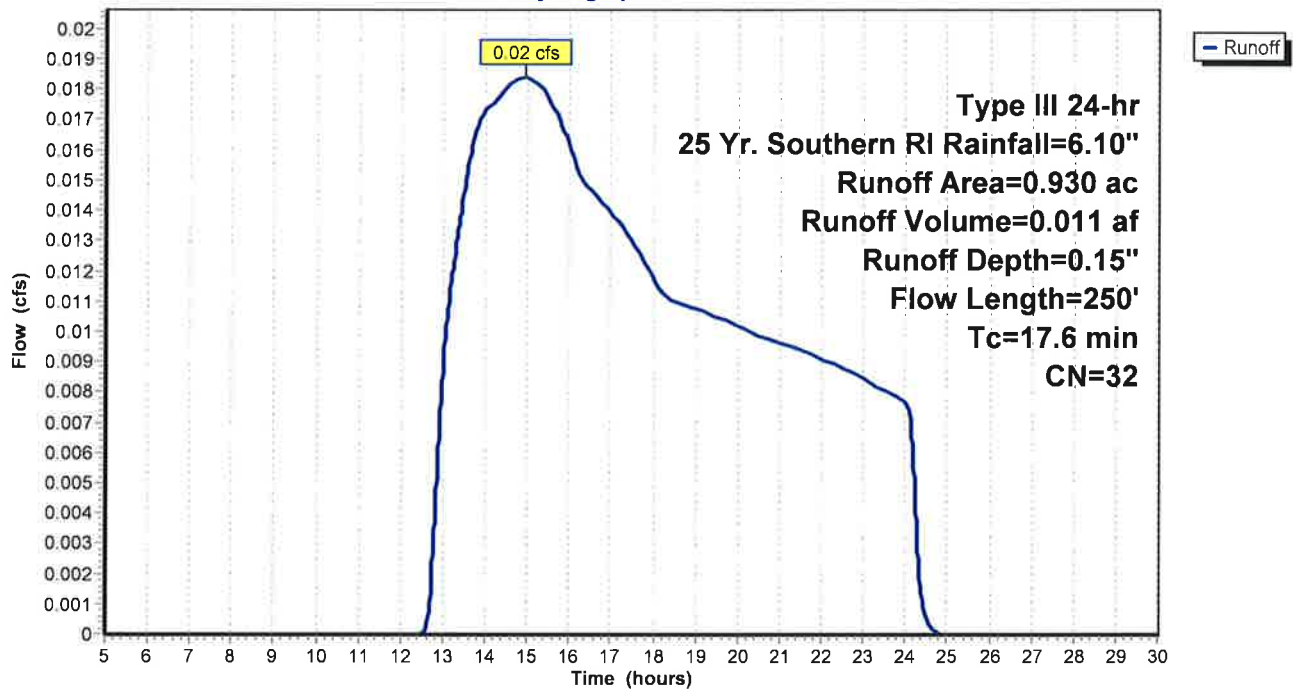
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

Area (ac)	CN	Description
0.700	30	Woods, Good, HSG A
0.230	39	>75% Grass cover, Good, HSG A
0.930	32	Weighted Average
0.930		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.2200	0.12		<b>Sheet Flow, Tt1</b>
					Woods: Dense underbrush n= 0.800 P2= 3.40"
3.7	150	0.0730	0.68		<b>Shallow Concentrated Flow, Tt2</b>
					Forest w/Heavy Litter Kv= 2.5 fps
17.6	250	Total			

**Subcatchment DA-2C: Drainage Area 2C**

Hydrograph



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Page 30

**Summary for Pond 1P: Swale-North Side**

Inflow Area = 1.930 ac, 6.22% Impervious, Inflow Depth = 0.37" for 25 Yr. Southern RI event  
 Inflow = 0.14 cfs @ 13.17 hrs, Volume= 0.059 af  
 Outflow = 0.10 cfs @ 14.57 hrs, Volume= 0.059 af, Atten= 27%, Lag= 83.7 min  
 Discarded = 0.09 cfs @ 14.57 hrs, Volume= 0.059 af  
 Primary = 0.01 cfs @ 14.57 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 103.46' @ 14.57 hrs Surf.Area= 450 sf Storage= 259 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 26.7 min ( 1,044.3 - 1,017.5 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

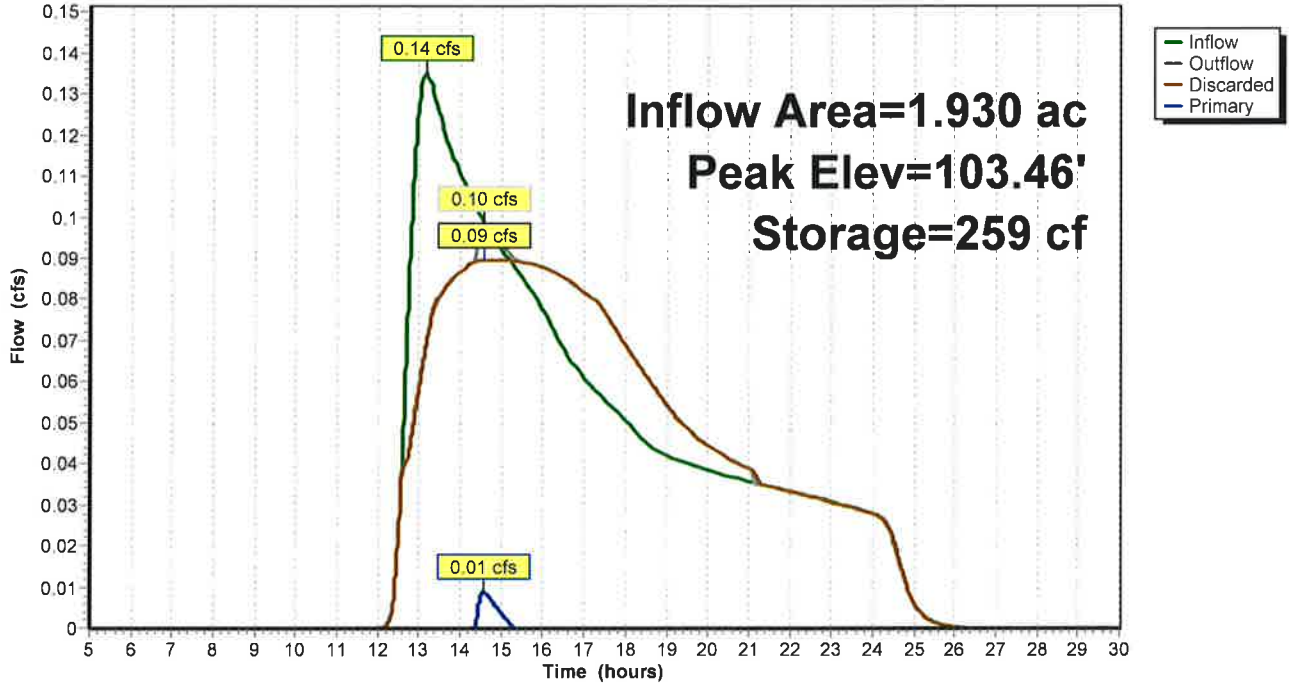
Device	Routing	Invert	Outlet Devices											
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'											
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32											

**Discarded OutFlow** Max=0.09 cfs @ 14.57 hrs HW=103.46' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.09 cfs)

**Primary OutFlow** Max=0.01 cfs @ 14.57 hrs HW=103.46' TW=96.75' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 0.01 cfs @ 0.23 fps)

**Pond 1P: Swale-North Side**

Hydrograph



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Page 32

**Summary for Pond 2P: Swale-South Side**

Inflow Area = 0.930 ac, 12.90% Impervious, Inflow Depth = 0.59" for 25 Yr. Southern RI event  
 Inflow = 0.18 cfs @ 12.73 hrs, Volume= 0.046 af  
 Outflow = 0.09 cfs @ 13.67 hrs, Volume= 0.046 af, Atten= 49%, Lag= 56.3 min  
 Discarded = 0.09 cfs @ 13.67 hrs, Volume= 0.046 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 103.44' @ 13.67 hrs Surf.Area= 447 sf Storage= 251 cf

Plug-Flow detention time= 23.1 min calculated for 0.046 af (100% of inflow)  
 Center-of-Mass det. time= 23.0 min ( 992.4 - 969.3 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

Device	Routing	Invert	Outlet Devices											
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'											
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32											

**Discarded OutFlow** Max=0.09 cfs @ 13.67 hrs HW=103.44' (Free Discharge)

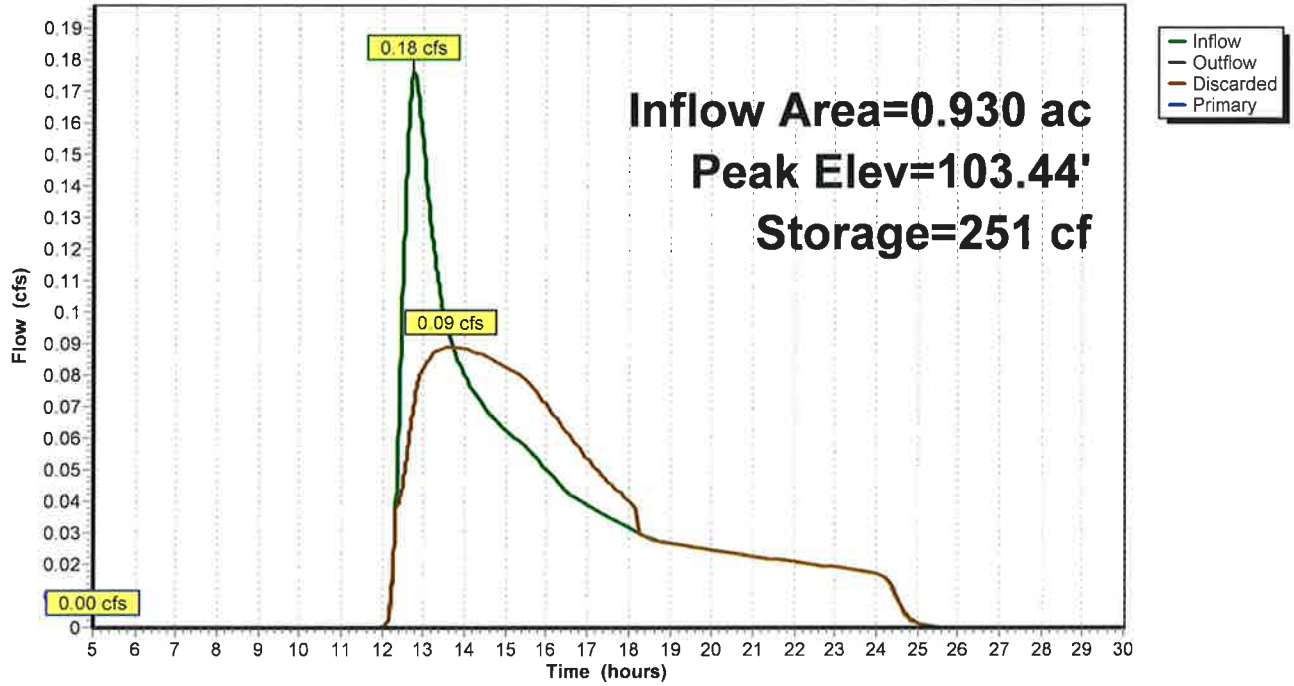
↑1=Exfiltration ( Controls 0.09 cfs)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=102.70' TW=96.75' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 2P: Swale-South Side

Hydrograph



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Page 34

**Summary for Pond 3P: Infiltration Trench**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 25 Yr. Southern RI event  
 Inflow = 0.01 cfs @ 14.57 hrs, Volume= 0.000 af  
 Outflow = 0.01 cfs @ 14.57 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min  
 Discarded = 0.01 cfs @ 14.57 hrs, Volume= 0.000 af  
 Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 96.75' @ 5.00 hrs Surf.Area= 108 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 0.0 min ( 883.8 - 883.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	96.75'	133 cf	<b>6.00'W x 18.00'L x 5.25'H Prismaoid</b> 567 cf Overall - 236 cf Embedded = 331 cf x 40.0% Voids
#2	97.75'	177 cf	<b>Concrete Galley 4x4x4 x 4 Inside #1</b> Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf
#3	102.00'	1,728 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>
		2,038 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
102.00	300	0	0	300
102.50	385	171	171	391
103.00	520	225	396	531
103.35	620	199	595	636
104.00	3,200	1,133	1,728	3,217

Device	Routing	Invert	Outlet Devices
#1	Discarded	96.75'	<b>12.000 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'
#2	Primary	103.50'	<b>5.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Discarded OutFlow** Max=0.00 cfs @ 14.57 hrs HW=96.75' (Free Discharge)

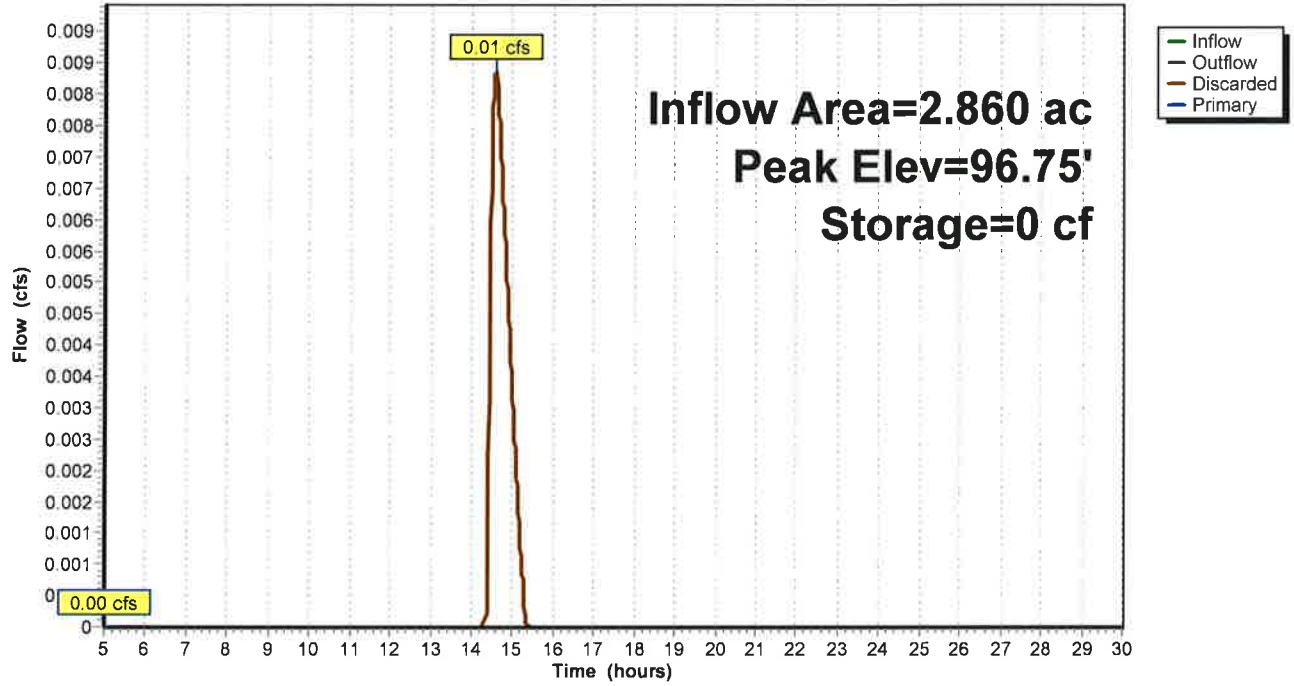
↑1=Exfiltration (Passes 0.00 cfs of 0.03 cfs potential flow)

**Primary OutFlow** Max=0.00 cfs @ 5.00 hrs HW=96.75' TW=0.00' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

### Pond 3P: Infiltration Trench

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 25 Yr. Southern RI Rainfall=6.10"

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Page 36

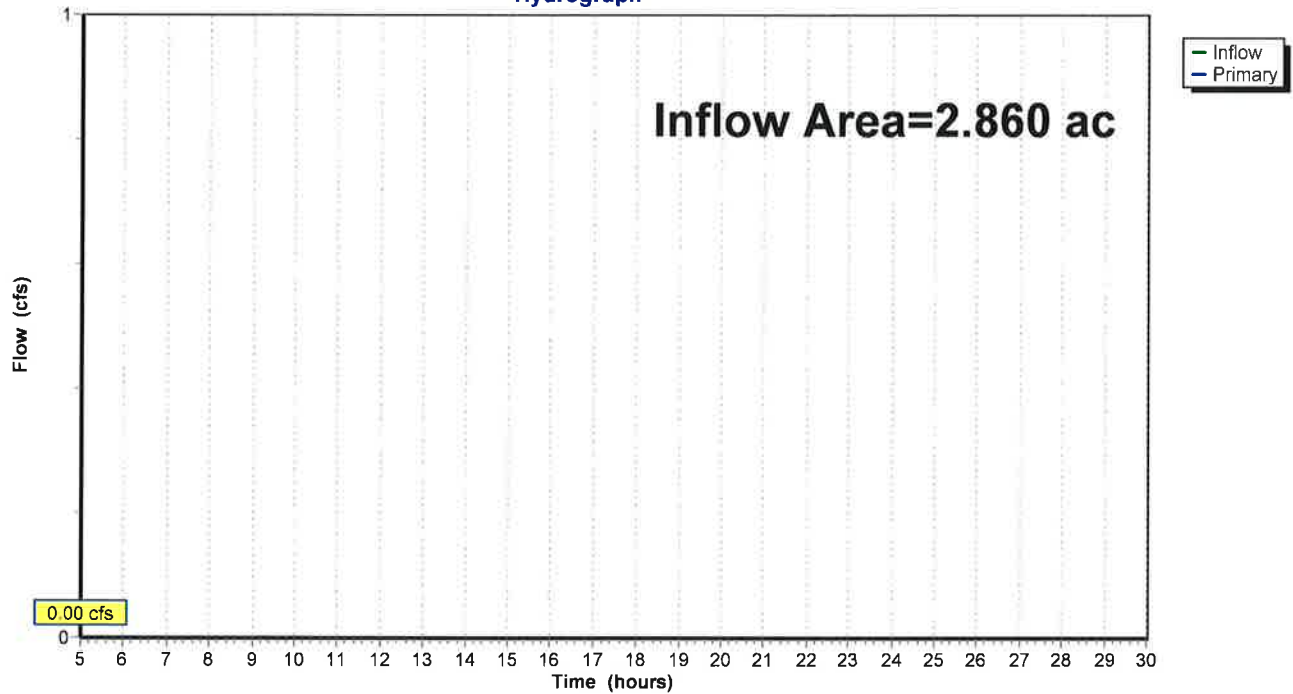
**Summary for Link 1L: Total Offsite**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.00" for 25 Yr. Southern RI event  
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af  
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

Hydrograph



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Page 37

**Summary for Subcatchment DA-1: Drainage Area 1**

Runoff = 1.47 cfs @ 12.43 hrs, Volume= 0.260 af, Depth= 0.98"

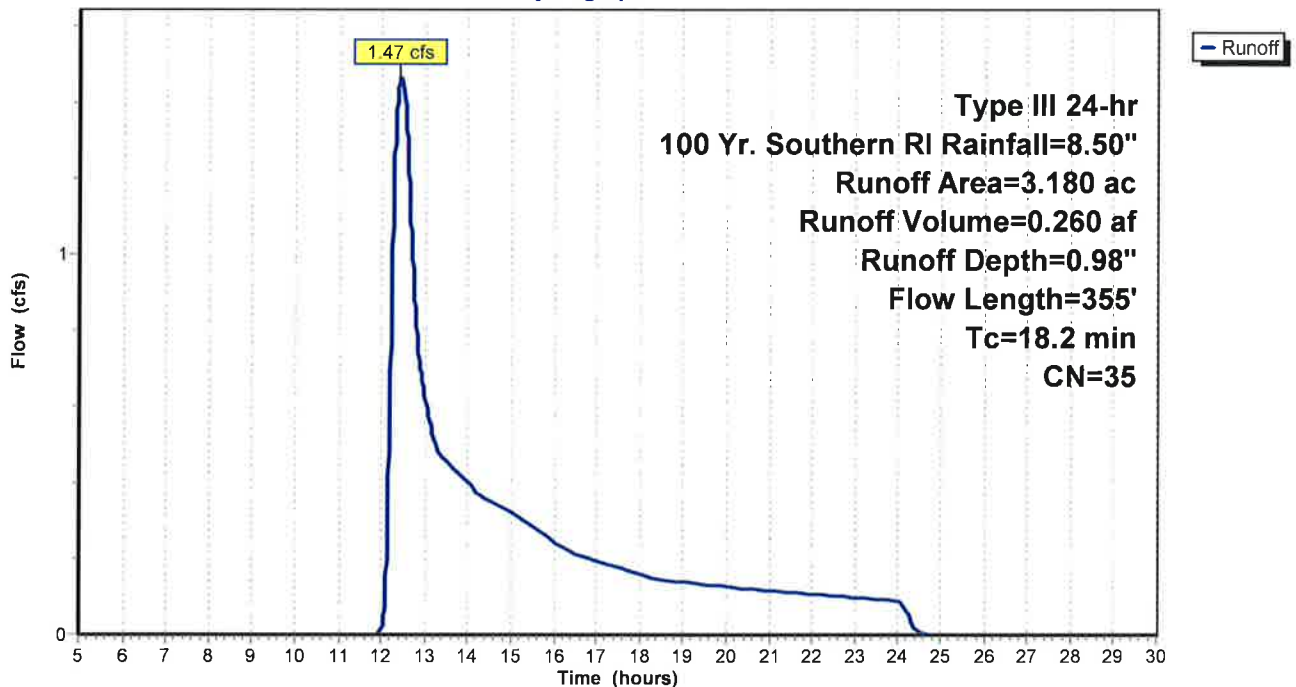
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

Area (ac)	CN	Description
2.430	30	Woods, Good, HSG A
0.080	98	Paved parking, HSG A
0.050	98	Roofs, HSG A
0.620	39	>75% Grass cover, Good, HSG A
3.180	35	Weighted Average
3.050		95.91% Pervious Area
0.130		4.09% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.8	150	0.3600	0.16		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.5	80	0.2500	2.50		<b>Shallow Concentrated Flow, Tt2</b> Woodland Kv= 5.0 fps
1.9	125	0.0480	1.10		<b>Shallow Concentrated Flow, Tt3</b> Woodland Kv= 5.0 fps
18.2	355	Total			

**Subcatchment DA-1: Drainage Area 1**

Hydrograph



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Page 38

**Summary for Subcatchment DA-2A: Drainage Area 2A**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.78 cfs @ 12.89 hrs, Volume= 0.189 af, Depth= 1.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

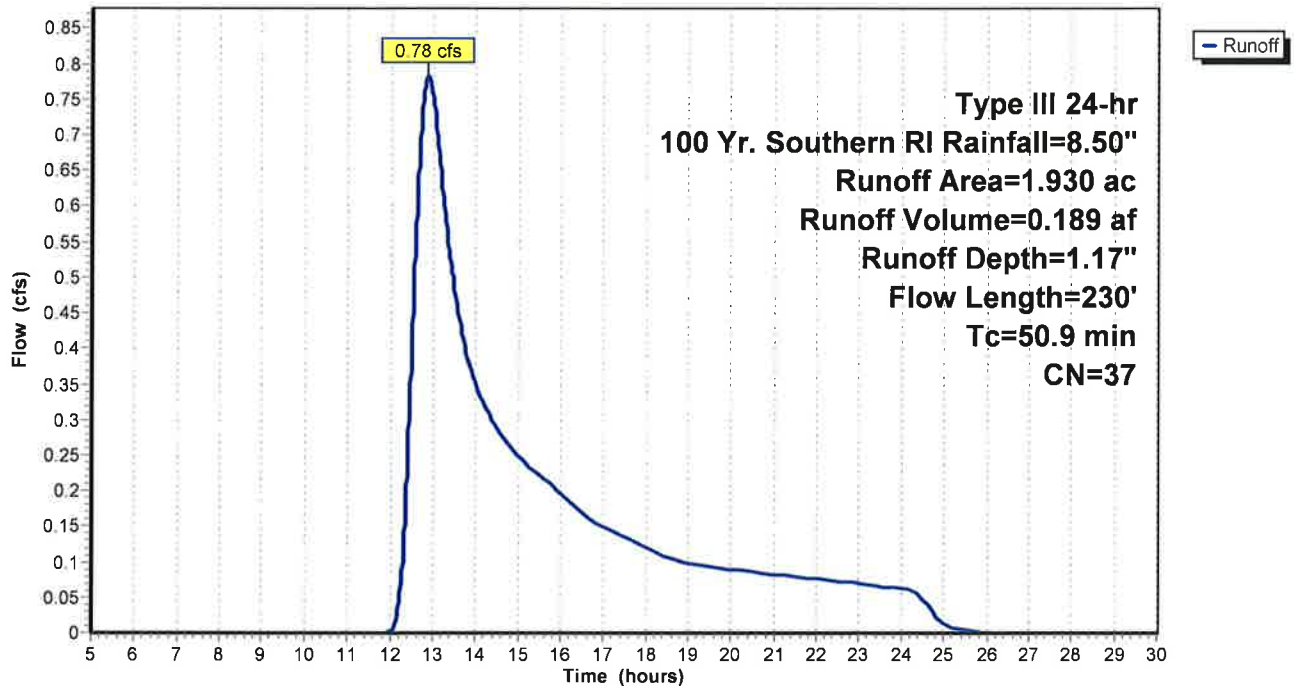
Area (ac)	CN	Description
1.200	30	Woods, Good, HSG A
0.610	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
1.930	37	Weighted Average
1.810		93.78% Pervious Area
0.120		6.22% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	120	0.0330	0.06		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
1.6	110	0.0270	1.15		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
50.9	230	Total			

**Subcatchment DA-2A: Drainage Area 2A**

Hydrograph



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Page 39

**Summary for Subcatchment DA-2B: Drainage Area 2B**

A direct input of a Tc of 12 minutes was utilized to account for check dams within the grassed swales

Runoff = 0.69 cfs @ 12.64 hrs, Volume= 0.122 af, Depth= 1.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

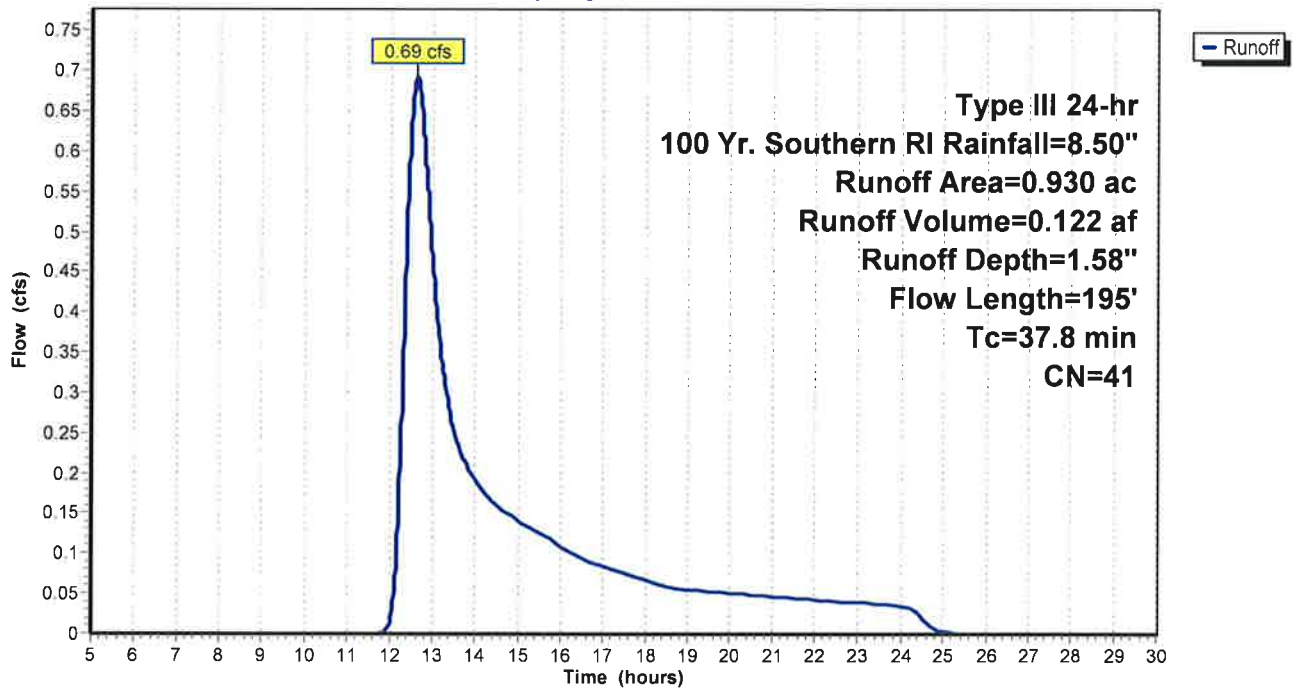
Area (ac)	CN	Description
0.600	30	Woods, Good, HSG A
0.210	39	>75% Grass cover, Good, HSG A
0.120	98	Paved parking, HSG A
0.930	41	Weighted Average
0.810		87.10% Pervious Area
0.120		12.90% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
22.2	130	0.1150	0.10		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
0.6	65	0.0600	1.71		<b>Shallow Concentrated Flow, Tt2</b> Short Grass Pasture Kv= 7.0 fps
15.0					<b>Direct Entry, Tt3-5</b>
37.8	195	Total			

**Subcatchment DA-2B: Drainage Area 2B**

Hydrograph



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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

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Page 40

**Summary for Subcatchment DA-2C: Drainage Area 2C**

Runoff = 0.25 cfs @ 12.50 hrs, Volume= 0.055 af, Depth= 0.71"

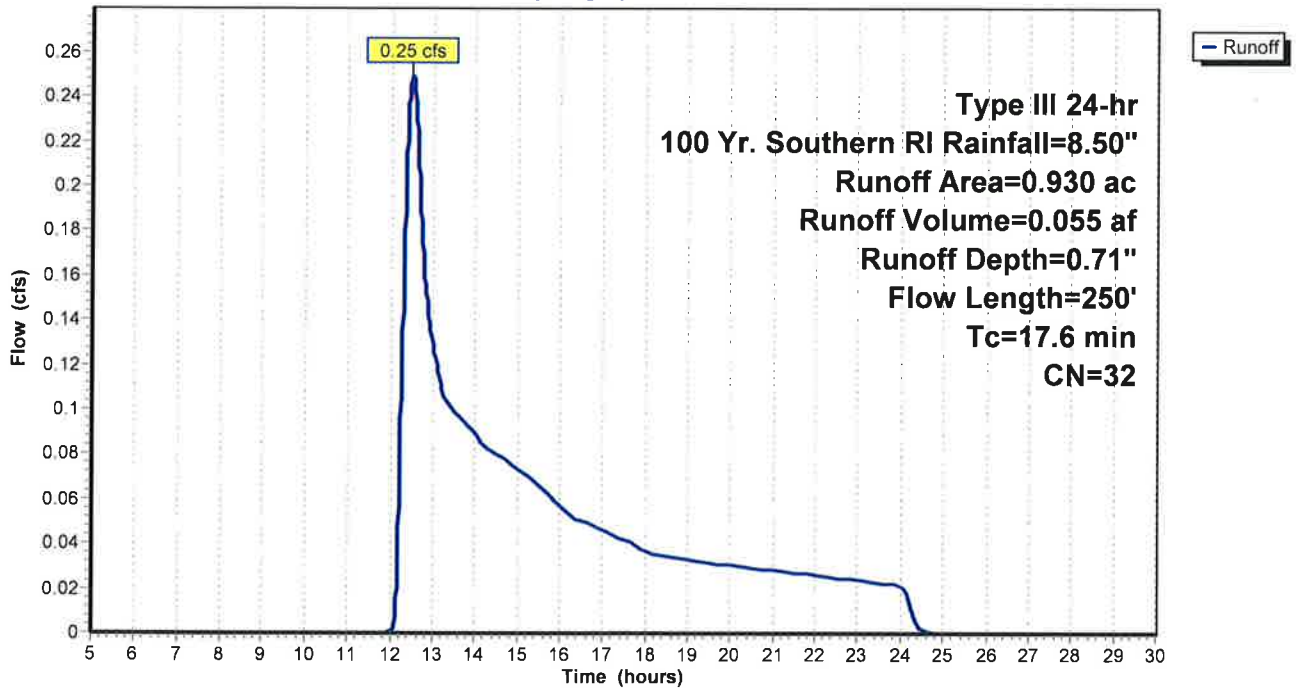
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

Area (ac)	CN	Description
0.700	30	Woods, Good, HSG A
0.230	39	>75% Grass cover, Good, HSG A
0.930	32	Weighted Average
0.930		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.9	100	0.2200	0.12		<b>Sheet Flow, Tt1</b> Woods: Dense underbrush n= 0.800 P2= 3.40"
3.7	150	0.0730	0.68		<b>Shallow Concentrated Flow, Tt2</b> Forest w/Heavy Litter Kv= 2.5 fps
17.6	250	Total			

**Subcatchment DA-2C: Drainage Area 2C**

Hydrograph



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Page 41

**Summary for Pond 1P: Swale-North Side**

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=15)

Inflow Area = 1.930 ac, 6.22% Impervious, Inflow Depth = 1.17" for 100 Yr. Southern RI event  
 Inflow = 0.78 cfs @ 12.89 hrs, Volume= 0.189 af  
 Outflow = 0.78 cfs @ 12.84 hrs, Volume= 0.189 af, Atten= 1%, Lag= 0.0 min  
 Discarded = 0.10 cfs @ 13.03 hrs, Volume= 0.093 af  
 Primary = 0.68 cfs @ 12.84 hrs, Volume= 0.095 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 103.64' @ 13.03 hrs Surf.Area= 488 sf Storage= 346 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 22.2 min ( 979.0 - 956.8 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

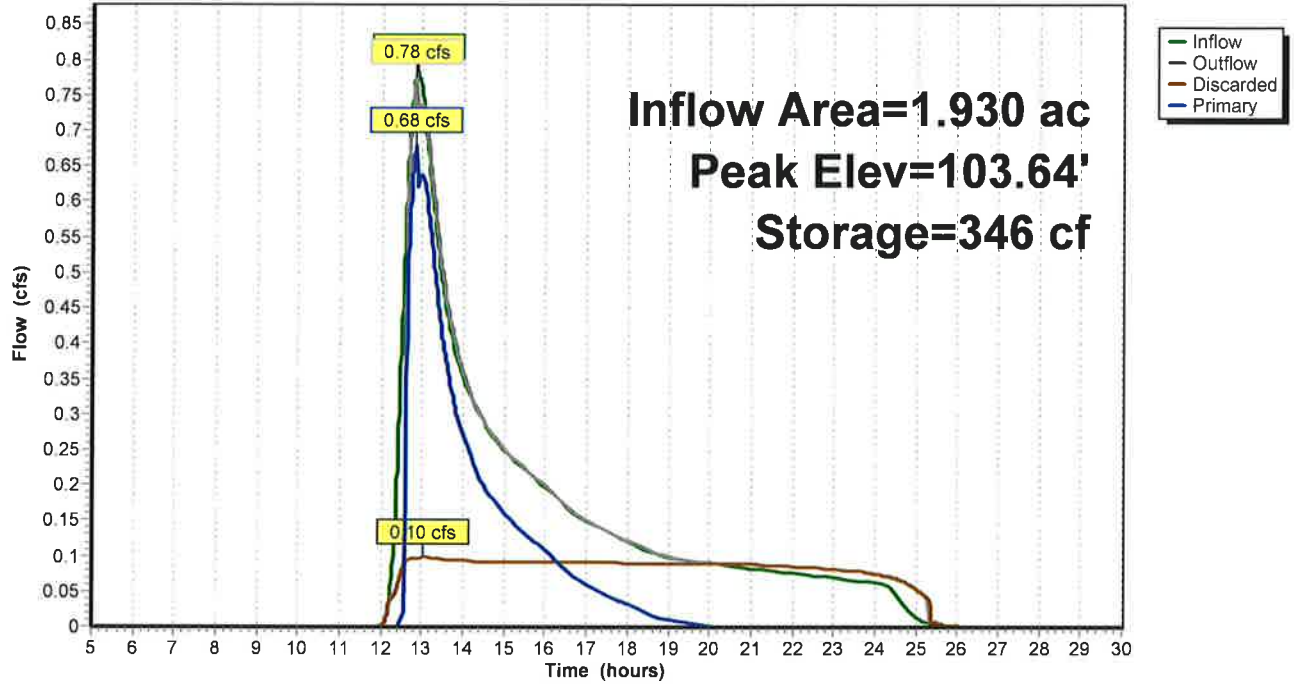
Device	Routing	Invert	Outlet Devices										
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'										
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32										

**Discarded OutFlow** Max=0.10 cfs @ 13.03 hrs HW=103.64' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.10 cfs)

**Primary OutFlow** Max=0.66 cfs @ 12.84 hrs HW=103.59' TW=103.47' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 0.66 cfs @ 0.93 fps)

**Pond 1P: Swale-North Side**

Hydrograph



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Page 43

**Summary for Pond 2P: Swale-South Side**

Inflow Area = 0.930 ac, 12.90% Impervious, Inflow Depth = 1.58" for 100 Yr. Southern RI event  
 Inflow = 0.69 cfs @ 12.64 hrs, Volume= 0.122 af  
 Outflow = 0.69 cfs @ 12.65 hrs, Volume= 0.122 af, Atten= 0%, Lag= 0.8 min  
 Discarded = 0.10 cfs @ 13.04 hrs, Volume= 0.074 af  
 Primary = 0.60 cfs @ 12.65 hrs, Volume= 0.049 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 103.62' @ 13.04 hrs Surf.Area= 483 sf Storage= 335 cf

Plug-Flow detention time= 21.8 min calculated for 0.122 af (100% of inflow)  
 Center-of-Mass det. time= 21.8 min ( 947.3 - 925.5 )

Volume	Invert	Avail.Storage	Storage Description		
#1	102.70'	374 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>		
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
102.70	200	0	0	200	
102.95	300	62	62	301	
103.20	400	87	149	402	
103.70	500	225	374	509	

Device	Routing	Invert	Outlet Devices											
#1	Discarded	102.70'	<b>8.300 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'											
#2	Primary	103.45'	<b>5.0' long x 2.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 Coef. (English) 2.54 2.61 2.61 2.60 2.66 2.70 2.77 2.89 2.88 2.85 3.07 3.20 3.32											

**Discarded OutFlow** Max=0.10 cfs @ 13.04 hrs HW=103.62' (Free Discharge)

↑1=Exfiltration ( Controls 0.10 cfs)

**Primary OutFlow** Max=0.60 cfs @ 12.65 hrs HW=103.58' TW=102.29' (Dynamic Tailwater)

↑2=Broad-Crested Rectangular Weir (Weir Controls 0.60 cfs @ 0.92 fps)

**Hemmerle Trust-Post Dev**

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Post-Development Drainage Analysis-350 Ministerial Rd

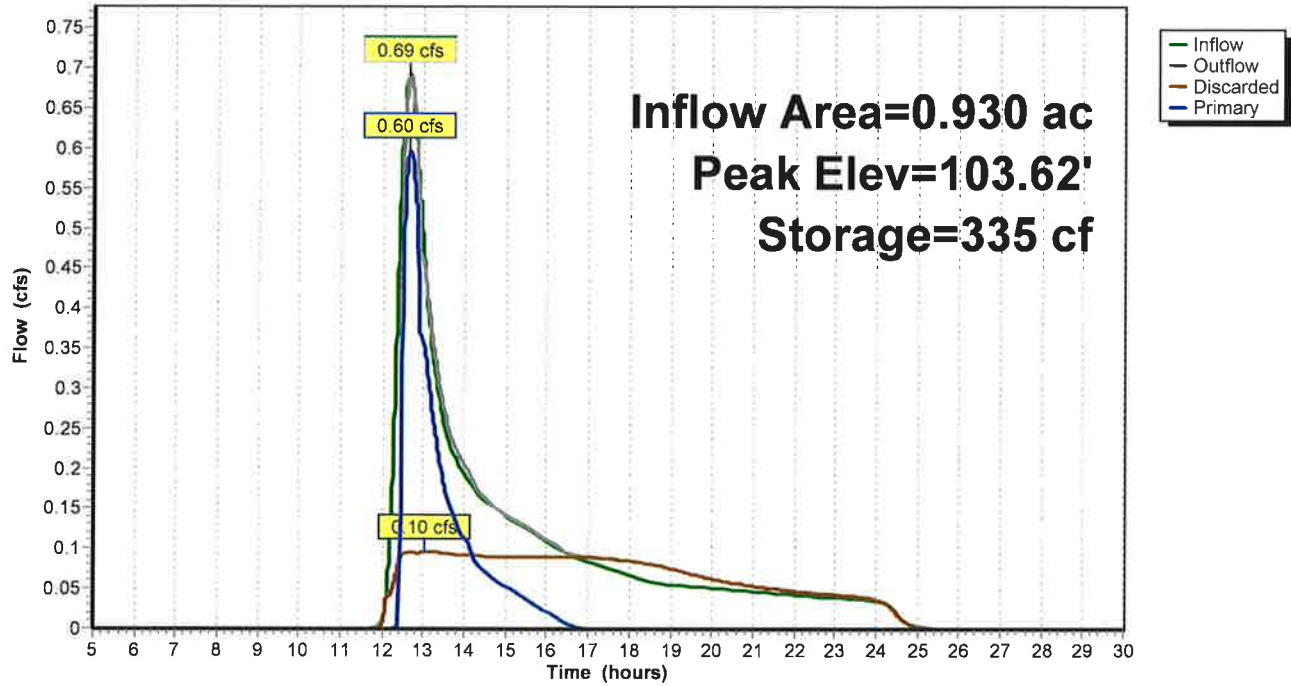
Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

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Page 44

**Pond 2P: Swale-South Side**

Hydrograph



**Hemmerle Trust-Post Dev**

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Page 45

**Summary for Pond 3P: Infiltration Trench**

[87] Warning: Oscillations may require smaller dt or Finer Routing (severity=30)

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.60" for 100 Yr. Southern RI event  
 Inflow = 1.20 cfs @ 12.78 hrs, Volume= 0.144 af  
 Outflow = 0.97 cfs @ 13.05 hrs, Volume= 0.144 af, Atten= 19%, Lag= 16.1 min  
 Discarded = 0.54 cfs @ 13.05 hrs, Volume= 0.126 af  
 Primary = 0.43 cfs @ 13.05 hrs, Volume= 0.018 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs  
 Peak Elev= 103.61' @ 13.05 hrs Surf.Area= 1,523 sf Storage= 1,164 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)  
 Center-of-Mass det. time= 34.0 min ( 868.8 - 834.8 )

Volume	Invert	Avail.Storage	Storage Description
#1	96.75'	133 cf	<b>6.00'W x 18.00'L x 5.25'H Prismaoid</b> 567 cf Overall - 236 cf Embedded = 331 cf x 40.0% Voids
#2	97.75'	177 cf	<b>Concrete Galley 4x4x4 x 4 Inside #1</b> Inside= 42.0"W x 43.0"H => 12.67 sf x 3.50'L = 44.3 cf Outside= 52.8"W x 48.0"H => 14.72 sf x 4.00'L = 58.9 cf
#3	102.00'	1,728 cf	<b>Custom Stage Data (Conic) Listed below (Recalc)</b>
		2,038 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
102.00	300	0	0	300
102.50	385	171	171	391
103.00	520	225	396	531
103.35	620	199	595	636
104.00	3,200	1,133	1,728	3,217

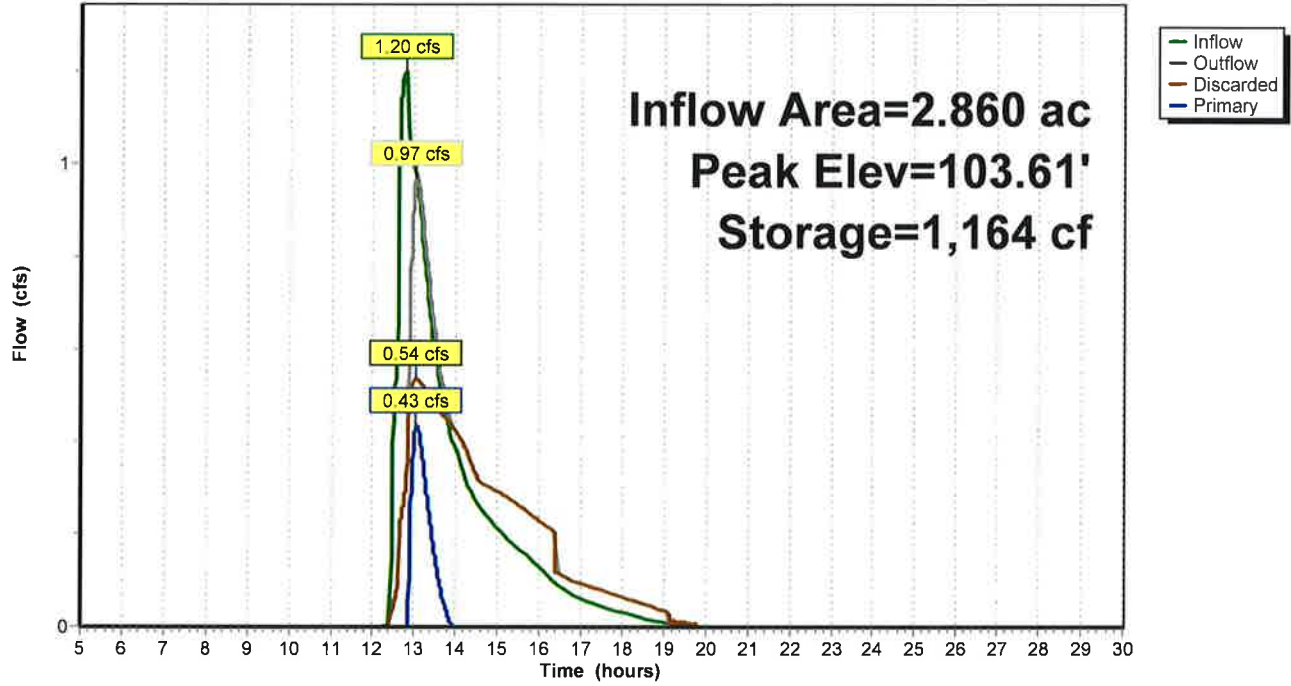
Device	Routing	Invert	Outlet Devices
#1	Discarded	96.75'	<b>12.000 in/hr Exfiltration over Wetted area</b> Conductivity to Groundwater Elevation = 80.00'
#2	Primary	103.50'	<b>5.0' long x 5.0' breadth Broad-Crested Rectangular Weir</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

**Discarded OutFlow** Max=0.54 cfs @ 13.05 hrs HW=103.61' (Free Discharge)  
 ↑1=Exfiltration ( Controls 0.54 cfs)

**Primary OutFlow** Max=0.43 cfs @ 13.05 hrs HW=103.61' TW=0.00' (Dynamic Tailwater)  
 ↑2=Broad-Crested Rectangular Weir (Weir Controls 0.43 cfs @ 0.78 fps)

### Pond 3P: Infiltration Trench

Hydrograph



**Hemmerle Trust-Post Dev**

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Post-Development Drainage Analysis-350 Ministerial Rd

Type III 24-hr 100 Yr. Southern RI Rainfall=8.50"

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Page 47

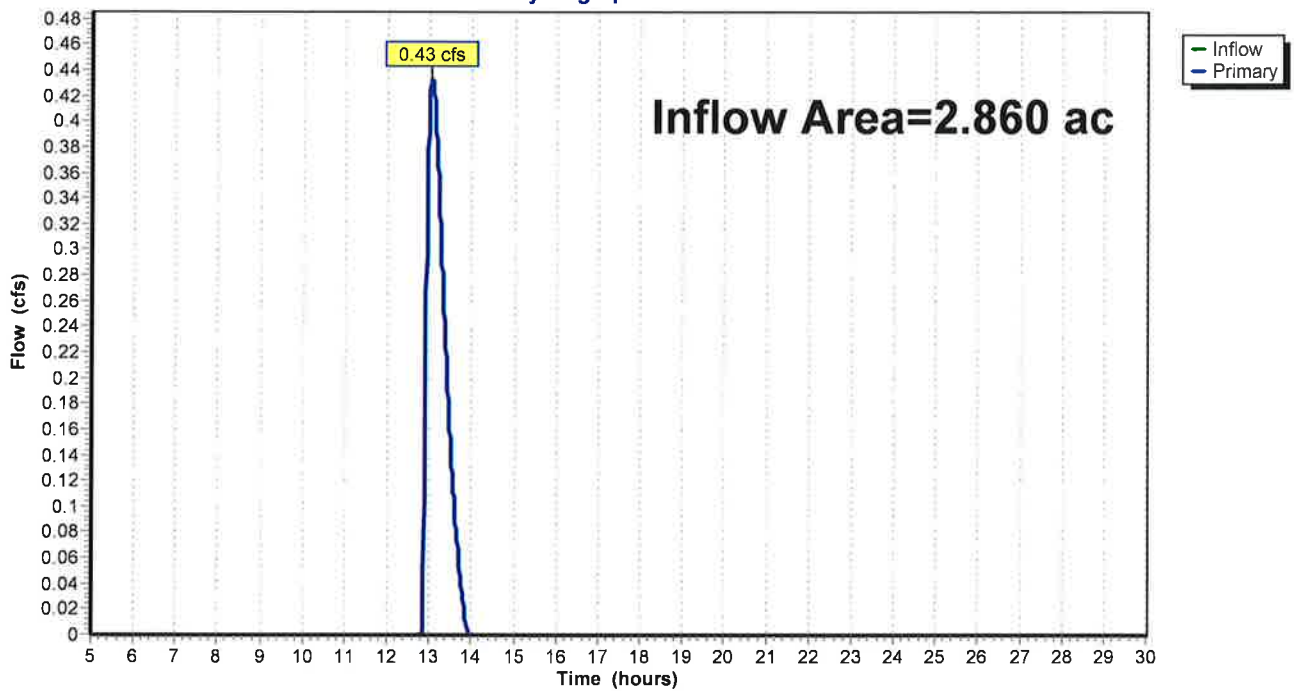
**Summary for Link 1L: Total Offsite**

Inflow Area = 2.860 ac, 8.39% Impervious, Inflow Depth = 0.08" for 100 Yr. Southern RI event  
Inflow = 0.43 cfs @ 13.05 hrs, Volume= 0.018 af  
Primary = 0.43 cfs @ 13.05 hrs, Volume= 0.018 af, Atten= 0%, Lag= 0.0 min

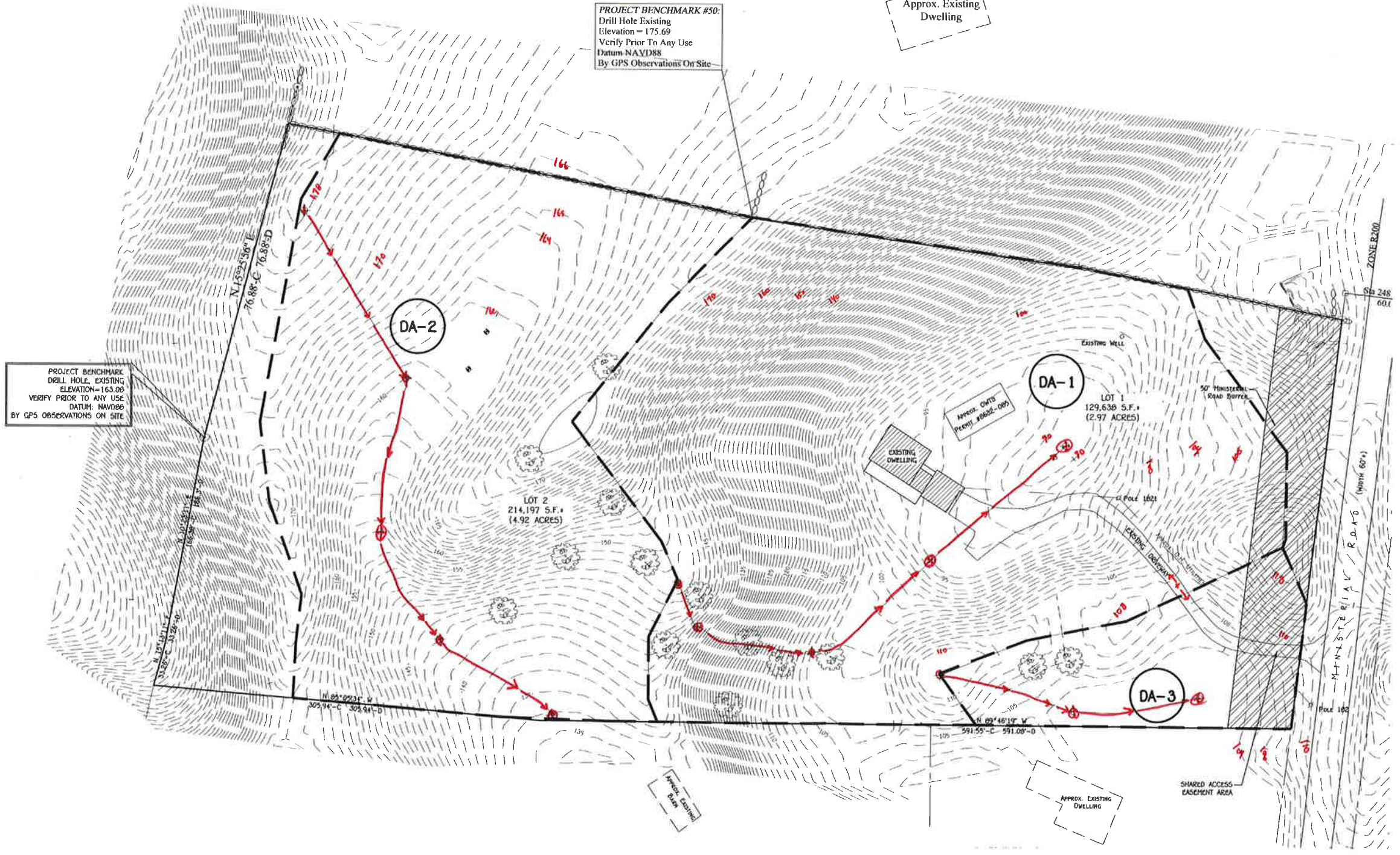
Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.01 hrs

**Link 1L: Total Offsite**

Hydrograph



**ATTACHMENT 4**  
**DRAINAGE AREA MAPS**



PROJECT BENCHMARK #50:  
 Drill Hole Existing  
 Elevation = 175.69  
 Verify Prior To Any Use  
 Datum NAD88  
 By GPS Observations On Site

Approx. Existing Dwelling

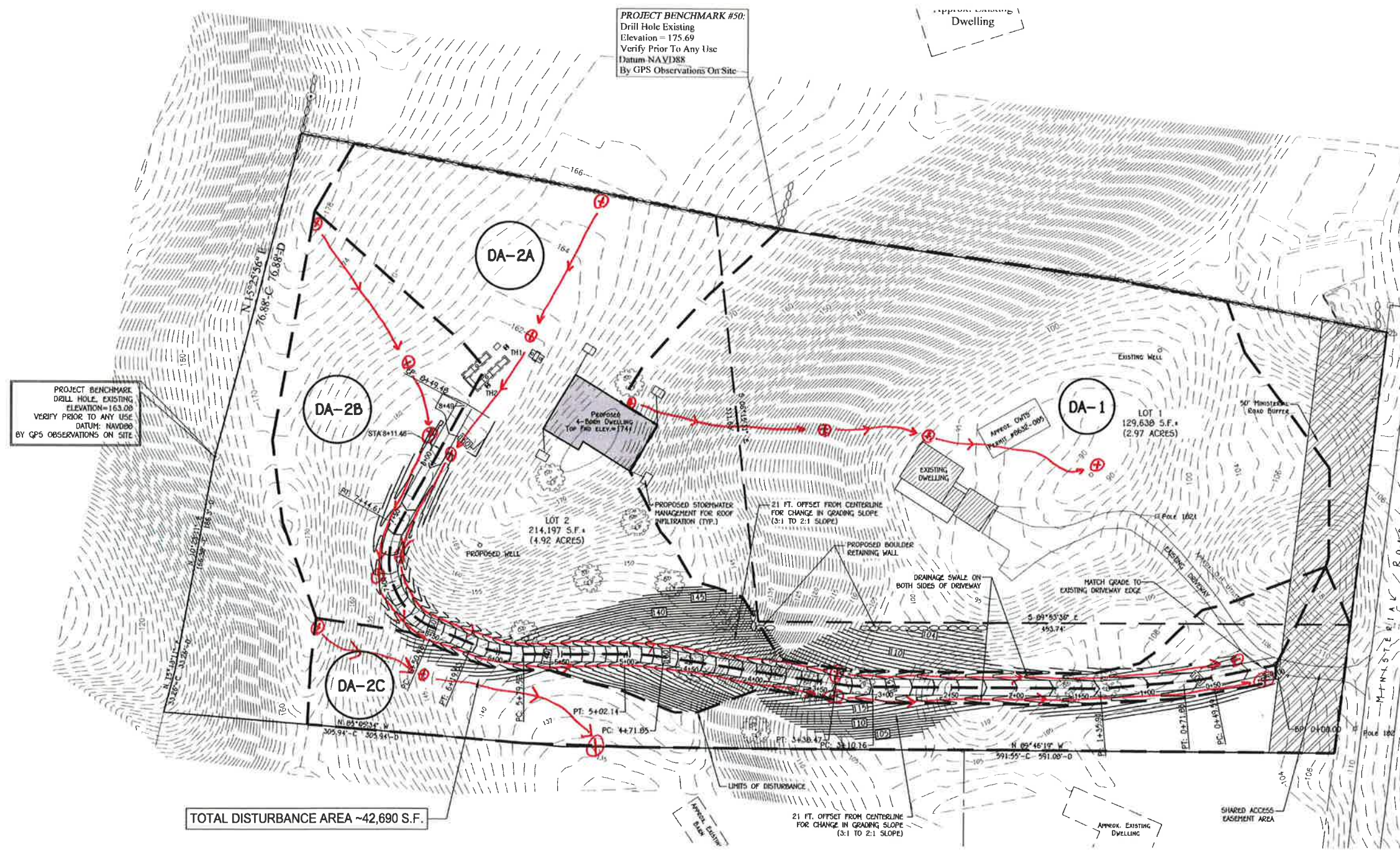
PROJECT BENCHMARK  
 DRILL HOLE, EXISTING  
 ELEVATION = 163.08  
 VERIFY PRIOR TO ANY USE  
 DATUM: NAVD88  
 BY GPS OBSERVATIONS ON SITE



PRE DEVELOPMENT DRAINAGE AREA MAP

PREPARED FOR  
 LEE G. HEMMERLE, TRUST  
 350 MINISTERIAL ROAD - PLAT 74, LOT 7  
 SOUTH KINGSTOWN, RHODE ISLAND

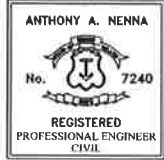
SCALE: 1"=40' APRIL 20, 2022 JOB I.D. NO. 21-032  
 PREPARED BY  
 ON-SITE ENGINEERING, INC.  
 3 CRESTVIEW DRIVE  
 WESTERLY, RHODE ISLAND 02891  
 TELE.: 401-348-6031



PROJECT BENCHMARK  
 DRILL HOLE, EXISTING  
 ELEVATION=163.09  
 VERIFY PRIOR TO ANY USE  
 DATUM: NAVD83  
 BY GPS OBSERVATIONS ON SITE

PROJECT BENCHMARK #50:  
 Drill Hole Existing  
 Elevation = 175.69  
 Verify Prior To Any Use  
 Datum NAVD88  
 By GPS Observations On Site

TOTAL DISTURBANCE AREA ~42,690 S.F.



POST DEVELOPMENT DRAINAGE AREA MAP

PREPARED FOR  
 LEE G. HEMMERLE, TRUST  
 350 MINISTERIAL ROAD - PLAT 74, LOT 7  
 SOUTH KINGSTOWN, RHODE ISLAND  
 SCALE: 1"=40' APRIL 20, 2022 JOB I.D. NO. 21-032  
 PREPARED BY  
 ON-SITE ENGINEERING, INC.  
 3 CRESTVIEW DRIVE  
 WESTERLY, RHODE ISLAND 02891  
 TELE.: 401-348-6831